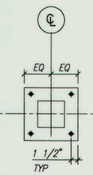

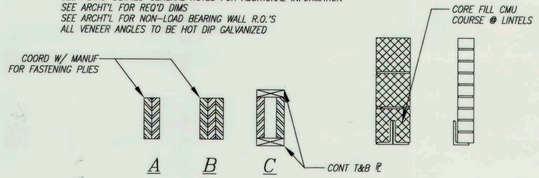

BASE PL SCHEDULE (BP)

TYPE	SIZE	DETAIL	AB'S	REMARKS
1	SIMPSON CB TYPE POST BASE			
2	PL 3/4" x 10" x 10"	A	(4)-3/4"ø	
3	PL 3/4" x 12" x 12"	A	(4)-3/4"ø	

A

BASE PLATE DETAILS

BEARING PL SCHEDULE (BRP)

TYPE	SIZE	DETAIL	A.B.'S	REMARKS
BRP1	PL 1/2 x 6-1/2 x 6-1/2	3/S7	(2) 3/4"	
BRP2	PL 3/4 x 7 x 10	3/S7	(2) 3/4"	
BRP3	PL 3/8 x 8 x 16	3/S7	(2) 3/4"	
BRP4	PL 3/8 x 8 x 8	3/S7	(2) 3/4"	

COLUMN FOOTING SCHEDULE

TYPE	SIZE	REINFORCING AT BOTT	REMARKS
F1	2'-6" x 2'-6" x 1'-0	(2)-#4 E.W. BOTT	
F2	3'-0 x 3'-0 x 1'-0	(3)-#4 E.W. BOTT	
F3	3'-6" x 3'-6" x 1'-0	(4)-#5 E.W. BOTT	
F4	4'-0 x 4'-0 x 1'-0	(5)-#5 E.W. BOTT	

HEADER / LINTEL SCHEDULE

TYPE	SIZE	DETAIL	MIN BRG EA END	REMARKS
H1	(2) 1.75" x 7.25" 1.3E TIMBERSTRAND (LSL)	A/C	3"	
H2	(2) 1.75" x 11.875" 1.5E TIMBERSTRAND (LSL)	A/C	6"	
H3	(2) 1.75" x 9.25" 1.5E TIMBERSTRAND (LSL)	A/C	3"	
H4	(3) 1.75" x 7.25" 1.3E TIMBERSTRAND (LSL)	B	3"	
MECH'L DUCT LINTEL	SIZE AS REQ'D TO SUIT MECH'L DUCT	G.C. TO COORD	8"	

MASONRY LINTELS

TYPE	SIZE	MAX M.O.	MIN BRG EA END	REMARKS
L1	(2) ∠6 x 3-1/2 x 1/4	4'-0"	8"	(6'-8" CONST M.O. @ ELEVATOR)
L2	∠4 x 4 x 1/4	4'-0"	8"	TYP @ EXTERIOR VENEER (U.N.)

NOTES
LINTELS ARE TO BE USED AT INDICATED AREAS ONLY (CONSULT ENGINEER FOR AREAS IN QUESTION)
SEE PLANS-DETAILS-GENERAL NOTES FOR ADDITIONAL INFORMATION
SEE ARCHT'L FOR REQ'D DIMS
SEE ARCHT'L FOR NON-LOAD BEARING WALL R.O.'S
ALL VENEER ANGLES TO BE HOT DIP GALVANIZED

LINTEL DETAILS

STRUCTURAL GENERAL NOTES

(UNLESS OTHERWISE NOTED ON DRAWINGS OR SPECIFICATIONS)

QUALITY ASSURANCE

THE FOLLOWING TESTING SHALL BE COMPLETED AND SUBMITTED TO THE (A/SER) FOR RECORD USE DURING CONSTRUCTION:

SOIL COMPACTION TEST RESULTS – BUILDING RELATED ***

PERFORM FIELD DENSITY TESTS IN ACCORDANCE WITH ASTM D-1557 AS FOLLOWS:

FOR EACH STRATA OF SOIL ON WHICH FOOTINGS WILL BE PLACED, CONDUCT AT LEAST ONE TEST TO VERIFY REQUIRED DESIGN BEARING CAPACITIES.

FOR BUILDING SLAB SUBGRADE AND AT EACH COMPACTED FILL LAYER, PERFORM AT LEAST ONE FIELD DENSITY TEST OF SUBGRADE FOR EVERY 2000 SQ.FT. OR LESS OF BUILDING SLAB AREA, BUT IN NO CASE LESS THAN 3 REPRESENTATIVE TESTS PER LAYER.

FOR FOUNDATION WALL BACKFILL PERFORM AT LEAST ONE FIELD DENSITY TEST FOR EACH 100 LINEAL FEET OR LESS OF WALL LENGTH, BUT IN NO CASE FEWER THAN 3 REPRESENTATIVE TESTS PER LAYER.

"***" INDICATES SUBMIT TO GEOTECHNICAL – SITE/CIVIL ENGINEER (G-S/CE) FOR REVIEW.

CONCRETE & GROUT TESTS (IN ACCORDANCE W/ ACI 318)

TESTING SHALL BE IN ACCORDANCE WITH ASTM AS FOLLOWS: SLUMP ASTM C 143, AIR CONTENT ASTM C 173, TEST SPECIMEN ASTM C 31, STRENGTH TESTS C 39. THESE TESTS SHALL BE PERFORMED RANDOMLY AT THE FOLLOWING TIMES:

(1) – MIN. DURING EACH FOOTING PLACEMENT
(1) – MIN. DURING EACH FOUNDATION WALL PLACEMENT
(1) – MIN. DURING SLAB PLACEMENT

OBTAIN (4)-SPECIMENS FOR EACH COMPRESSIVE TEST.
(1) TESTED AT 7 DAYS, (2) AT 28 DAYS AND ONE RETAINED FOR LATER TESTING IF REQUIRED.

MASONRY

TESTED AND INSPECTED AS NOTED WITHIN THE MASONRY SECTION OF THE GENERAL NOTES (SEE BELOW).

NOTE: IF ANY RESULTS ARE NOT IN CONFORMANCE WITH REQUIREMENTS THEY SHALL BE IMMEDIATELY REPORTED TO THE CONTRACTOR AND (A/SER).

FOUNDATIONS

ALL SOILS WORK SHALL BE IN COMPLIANCE WITH THE GEOTECHNICAL STUDY AS PREPARED BY M&W SOILS ENGINEERING, INC., DATED MARCH 9, 2000.

1. THE CONTRACTOR IS RESPONSIBLE FOR ASSURING THAT ALL COMPACTION, APPROVAL OF SOILS MATERIALS AND PREPARATION OF EARTH WORK IS IN ACCORDANCE WITH THE PROJECT SPECIFICATION AND COORDINATED WITH THE GEOTECHNICAL – SITE/CIVIL ENGINEER (G-S/CE) RECOMMENDATIONS. PROVIDE TEST RESULTS TO (A/SER) FOR RECORD USE PRIOR TO PLACING FORM WORK.
2. FOOTINGS AND SLABS HAVE BEEN DESIGNED TO BEAR UPON SOILS HAVING A MINIMUM BEARING CAPACITY OF 3500 P.S.F. AS DETERMINED BY THE GEOTECHNICAL ENGINEER (M&W SOILS ENGINEERING, INC.) WHO IS TO BE NOTIFIED TO CONFIRM CONDITIONS CONFORM WITH RECOMMENDATIONS. STATEMENT OF CONFORMANCE SHALL BE ISSUED TO THE OFFICE OF (A/SER) FOR RECORD USE PRIOR TO PLACEMENT OF FILL OR FORM WORK. FAILURE TO DO SO WILL RESULT IN A DISCLAIMER OF RESPONSIBILITY BY THE (SER).
3. THE STRUCTURAL DESIGN IS BASED UPON THE SITE BEING ADEQUATELY PREPARED AS DIRECTED BY THE OWNER'S (G-S/CE). FAILURE TO DO SO COULD PRESENT ADVERSE CONDITIONS FOR WHICH THE STRUCTURE WAS NOT DESIGNED. SPECIFICALLY, THE CONTROL OF GROUND AND SURFACE WATER BOTH DURING AND AFTER CONSTRUCTION AND THE PREPARATION OF SUBGRADE TO CONFORM WITH THE DESIGN BEARING CAPACITY. IF THESE ITEMS ARE NOT VERIFIED, THERE IS NO ASSURANCE THE STRUCTURE WILL PERFORM AS DESIGNED.
4. FOUNDATION WALLS & SLAB ON GRADE HAVE NOT BEEN DESIGNED FOR HYDROSTATIC PRESSURE, THEREFORE THE (G-S/CE) SHALL MAKE DESIGN PROVISIONS TO PREVENT THESE FORCES FROM OCCURRING AS REQUIRED.
5. (SER) MAY MODIFY FOOTINGS AS RECOMMENDED BY (G-S/CE), DEPENDANT ON SOILS CONDITIONS ENCOUNTERED DURING EXCAVATION.
6. NO DELETERIOUS MATERIALS SUCH AS ORGANIC MATERIALS, BOULDERS OR FROZEN MATERIALS SHALL BE PLACED IN FILLS OR BACKFILLS.
7. NO FOOTINGS OR SLABS SHALL BE PLACED ON FROZEN, SATURATED OR DELETERIOUS MATERIALS, PROTECT FROM FREEZING UNTIL FULLY BACKFILLED.
8. BACKFILL BOTH SIDES OF FOUNDATION WALLS EQUALLY. RETAINING WALLS WHICH ARE TO BE BACKFILLED PRIOR TO ATTAINING FULL DESIGN STRENGTH (IE 28 DAYS AFTER PLACEMENT) SHALL BE ADEQUATELY BRACED TO PREVENT MOVEMENT OR STRUCTURAL DAMAGE.
9. NO EXTERIOR FOOTING (WALL OR COLUMN) SHALL BE PLACED LESS THAN 4'-0" BELOW FINISHED GRADE.
10. EXCAVATIONS NEAR BOTTOM OF FOOTING ELEVATION (B.O.F.) SHALL BE DONE WITH A SMOOTH EDGE BUCKET OR BY HAND EXCAVATION TO PREVENT SURFACES FROM BECOMING MUDDY. ALSO, THE TOP LAYER OF ALL EXCAVATIONS SHALL NOT BE DISTURBED WHEN SOIL IS WET. MAINTAIN DE-WATERING OPERATIONS AS REQUIRED.
11. SUBGRADE EXPLORATION HAS BEEN LIMITED AS OUTLINED WITHIN GEOTECH REPORT. AT ANY OTHER LOCATIONS, THE (SER) MAKES NO REPRESENTATIONS CONCERNING THE SUITABILITY OF ANY SOIL OR LEDGE MATERIAL, NOR IS THE ABSENCE OF DELETERIOUS MATERIALS, EITHER NATURALLY OCCURRING OR FORMERLY BURIED, ASSURED.
12. THE BASES OF FOOTINGS SHALL BE SUCH THAT THEY DO NOT FALL WITHIN A 1:2 SLOPE FROM THE BASE OF ANY ADJACENT FOOTING.

CONCRETE & REINFORCING

1. ALL CONCRETE SHALL BE PLACED, CURED AND TESTED IN ACCORDANCE WITH ALL AMERICAN CONCRETE INSTITUTE (A.C.I.) SPECIFICATIONS, LATEST EDITION.
2. THE CONTRACTOR SHALL SUBMIT, MIX DESIGNS OF EACH TYPE OF CONCRETE TO BE USED, TO THE ENGINEER FOR HIS RECORDS. MIX DESIGN SUBMITTAL SHALL INCLUDE (2) TEST BREAK RESULTS OF REPRESENTATIVE SAMPLES OF EACH TYPE. OR THE CONTRACTOR SHALL SUBMIT LETTER FROM INDEPENDENT TESTING LABORATORY / PRODUCER CERTIFYING PROPOSED MIX WILL MEET REQUIRED DESIGN STRENGTH.
3. ALL CONCRETE SHALL BE STANDARD WEIGHT (145 P.C.F.) AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3000 P.S.I. WITHIN A 28 DAY PERIOD UNLESS NOTED OTHERWISE. MAXIMUM SLUMP 3" +/- 1".
4. ALL EXTERIOR SLABS TO BE 4000 P.S.I. AIR ENTRAINED CONCRETE (6% MIN) WITH WWM. MAXIMUM SLUMP 3" +/- 1".
5. ALL REINFORCING SHALL BE THE MINIMUM REQUIREMENTS OF, AND PLACED IN ACCORDANCE WITH ALL A.C.I. & CONCRETE REINFORCING STEEL INSTITUTE SPECIFICATIONS, LATEST EDITION (UNLESS OTHERWISE SHOWN).
6. ALL REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60 (FY = 60,000 P.S.I.) DEFORMED BARS, UNLESS NOTED OTHERWISE.
7. CLEAR CONCRETE PROTECTION FOR REINFORCING
   A. FOOTINGS .... 3"
   B. FOUNDATION WALLS .... 2"
   C. SLABS .... (SEE DRAWINGS)
   D. PIERS .... 2" TO TIES
8. PLACE (1)-#4X2'0" LONG DIAGONALLY AT EACH CORNER OF EACH FACE OF ALL OPENINGS THROUGH CONCRETE WALLS LARGER THAN 8"X8". PROVIDE SLEEVES AROUND ALL PENETRATIONS. COORDINATE WITH ENGINEER.
9. ALL VERTICAL PIER REINFORCING TO HAVE MATCHING DOWELS INTO FOOTINGS WITH MINIMUM LAP OF 30 BAR DIAMETERS.
10. ALL HORIZONTAL WALL REINFORCING SHALL BE CONTINUOUS AROUND CORNERS WITH MINIMUM LAP OF 24 BAR DIAMETERS, UNLESS OTHERWISE SHOWN/NOTED ON PLAN, (PROVIDE SET-UP BARS AS REQ'D).
11. ALL WELDED WIRE MESH (WWM) SHALL CONFORM TO ASTM A185. LAP TWO SQUARES AT ALL SIDES AND TIE @ 3'-0" O.C. (SEE DRAWINGS FOR SIZE). ALL MESH SHALL BE SUPPORTED ON CHAIRS OR BOLSTERS AS REQUIRED AND AS SHOWN OR NOTED ON THE DRAWINGS.
12. SLAB CONTROL JOINTS, NOT SHOWN ON DRAWINGS, SHALL BE LAID OUT IN A SQUARE OR RECTANGULAR FASHION WITH AN AREA NOT EXCEEDING 480 SQ. FT. AND WITH THE LENGTH NOT EXCEEDING THE WIDTH BY 20%.
    FILL JOINTS WITH SEMI-RIGID EPOXY JOINT FILLER SUCH AS METZGER/MCGUIRE SPAL-PRO XL OR EQUAL. NO ASSURANCE IS OFFERED BY THE ENGINEER THAT RANDOM SHRINKAGE CRACKING WILL NOT OCCUR. COORDINATE LOCATIONS WITH ARCHITECTS.
13. NO DYNAMIC CONCENTRATED CONSTRUCTION LOADS EXCEEDING THE ALLOWABLE UNIFORMLY DISTRIBUTED LIVE LOADS NOTED HEREIN ARE TO BE APPLIED TO ANY CONCRETE SLABS.
14. ALL TOP OF WALL ANCHOR BOLTS SHALL BE 1/2" DIAMETER THREADED BOLTS (NO SUBSTITUTES), 8" MIN. EMBEDMENT, SPACING AT 4'-0" O.C..
15. DO NOT CUT, NOTCH, CORE OR SLEEVE THROUGH FOOTINGS OR EMBED MECHANICAL / ELECTRICAL UTILITIES IN OR THROUGH FOOTINGS WITHOUT FIRST CONSULTING WITH THE ENGINEER, IN ADVANCE.

CONTROLLED LOW-STRENGTH MATERIAL (CLSM) "FLOWABLE FILL"

1. DESCRIPTION: THIS WORK SHALL CONSIST OF FURNISHING, TRANSPORTING, AND PLACING CONTROLLED LOW-STRENGTH MATERIAL (CLSM) AS BACKFILL IN TRENCHES OR AT OTHER LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.
2. MATERIALS: CLSM SHALL CONSIST OF A MIXTURE OF PORTLAND CEMENT, FLY ASH, FINE AGGREGATE, AND WATER PROPORTIONED TO PROVIDE A BACKFILL MATERIAL THAT IS SELF-COMPACTING AND CAPABLE OF BEING EXCAVATED WITH HAND TOOLS IF NECESSARY AT A LATER DATE.

   ALL MATERIALS SHALL MEET THE FOLLOWING REQUIREMENTS OF SECTION 700 OF THE STANDARD SPECIFICATIONS:

PORTLAND CEMENT, TYPES I	SECTION 701
WATER	SECTION 702
FINE AGGREGATE (NATURAL SAND)	SECTION 703.02
FLY ASH	SECTION 718.19(B)

3. PROPORTIONING: MATERIALS FOR CLSM SHALL BE PROPORTIONED AS FOLLOWS:

PORTLAND CEMENT	50 LBS.
FLY ASH	300 LBS.(IF TYPE F) OR 200 LBS.(IF TYPE C)
FINE AGGREGATE (SATURATED SURFACE DRY)	2900 LBS.
WATER	45-65 GALLONS

   THESE QUANTITIES WILL YIELD APPROXIMATELY ONE CUBIC YARD OF CLSM OF THE PROPER CONSISTENCY. THE FLOWABILITY SHALL BE OBSERVED BY THE ENGINEER AND THE WATER CONTENT ADJUSTED WITHIN THE SPECIFIED LIMITS TO PRODUCE DESIRED RESULTS. THE CLSM SHALL BE READY-MIXED AS SPECIFIED IN SECTION 504.11 OF THE STANDARD SPECIFICATIONS.

   SUFFICIENT MIXING CAPACITY SHALL BE PROVIDED TO PERMIT THE CLSM TO BE PLACED WITHOUT INTERRUPTION. THE MIXER DRUM SHALL BE COMPLETELY EMPTIED PRIOR TO THE INITIAL BATCH OF CLSM TO ENSURE THAT NO ADDITIONAL CEMENT LINES ARE INCORPORATED INTO THE MIX.
4. PLACEMENT: THE CLSM SHALL BE DISCHARGED DIRECTLY FROM THE TRUCK INTO THE SPACE TO BE FILLED, OR BY OTHER METHODS APPROVED BY THE ENGINEER. WHEN BACKFILLING PIPE CULVERTS, CLSM SHALL BE DISTRIBUTED EVENLY TO PRECLUDE ANY MOVEMENT OF THE PIPE. FOR OTHER THAN PORTLAND CEMENT CONCRETE PIPES, PLACEMENT WILL BE REQUIRED IN STAGES TO PREVENT UPLIFT OF THE PIPE CULVERT, THE FIRST STAGE PLACEMENT SHALL STOP AT ONE-FOURTH THE HEIGHT OF THE CULVERT. AFTER CONSOLIDATION OF THE FIRST LIFT, AS DETERMINED BY THE ENGINEER, THE SECOND STAGE PLACEMENT SHALL STOP MID-HEIGHT OF THE CULVERT. AFTER CONSOLIDATION OF THE SECOND LIFT, THE REMAINDER OF THE TRENCH SHALL BE FILLED IN ONE OPERATION.
5. LIMITATION OF OPERATIONS: CLSM SHALL NOT BE PLACED IN FROZEN GROUND. MIXING AND PLACING MAY BEGIN ONLY IF THE AIR TEMPERATURE IS AT LEAST 35 F AND RISING. AT TIME OF PLACEMENT, CLSM SHALL HAVE A TEMPERATURE OF AT LEAST 40 F. MIXING AND PLACING SHALL STOP WHEN THE AIR TEMPERATURE IS 40 F AND FALLING.

MASONRY

1. ALL REINFORCED CONCRETE MASONRY SHALL BE IN ACCORDANCE WITH ACI 530.1-95/ASCE 6-95/TMS 602-95 SPECIFICATIONS FOR MASONRY STRUCTURES (THE MASONRY 530 CODE).
   ALL SAMPLING AND TESTING SHALL BE IN ACCORDANCE W/ ASTM.
2. ALL CONCRETE MASONRY UNITS (CMU) SHALL BE 2 CORE NORMAL WEIGHT BLOCK (125 P.C.F. MIN.) EXCEPT WHERE NOTED.
3. ALL MORTAR SHALL CONFORM TO ASTM C270, TYPE S MORTAR (1800 P.S.I.) AT ALL EXTERIOR AND LOAD BEARING WALLS.
4. ALL GROUT SHALL CONFORM TO ASTM C476, AND SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 3000 P.S.I. (TESTED IN ACCORDANCE W/ ASTM C1019).
5. HOLLOW LOAD BEARING CMU SHALL CONFORM TO ASTM C90, TYPE 1, GRADE N-1 (TESTED IN ACCORDANCE W/ ASTM C140).
   CMU SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 2000 P.S.I.
   CMU ASSEMBLY SHALL HAVE A MIN. COMPRESSIVE STRENGTH (FM') OF 1500 P.S.I.
6. HORIZONTAL JOINT REINFORCEMENT SHALL BE LADDER TYPE, CONSISTING OF 9 GA. GALVANIZED WIRE WITH A SINGLE PAIR OF DEFORMED SIDE RODS AND CROSSRODS IN CONFORMANCE WITH ASTM A-82, INSTALLED IN ALTERNATE COURSES (16" O.C.). PROVIDE PRE-FORMED TEES AND CORNERS AT ALL WALL INTERSECTIONS. PROVIDE Z BARS AS SHOWN.
7. VERTICAL REINFORCEMENT SHALL CONSIST OF GRADE 60 DEFORMED BARS IN CONFORMANCE WITH ASTM A615.
8. ALL VERTICAL REINFORCING TO BE DOWELLED INTO FOUNDATION WALL WITH MINIMUM LAP AND EMBEDMENT OF 30 BAR DIAMETERS (UNLESS NOTED).
9. ALL VERTICAL REINFORCING TO BE CONTINUOUS FULL HEIGHT WITH MINIMUM LAP OF 30 BAR DIAMETERS (UNLESS NOTED).
10. ALL CORES WITH VERTICAL REINFORCING TO BE COMPLETELY FILLED WITH 3000 P.S.I. GROUT. COORDINATE METHOD OF GROUTING (HI-LIFT, LO-LIFT) WITH FABRICATION OF VERTICAL REINFORCING.
11. PROVIDE (1)-#6 VERTICALLY AT EACH JAMB OF WINDOWS AND DOORS (UNLESS NOTED OTHERWISE).
12. PROVIDE (1)-#6 VERTICALLY AT ALL BEAM BEARING LOCATIONS CONTINUOUS TO FOUNDATION (UNLESS NOTED OTHERWISE).
13. PROVIDE #4 @ 48" O.C. VERTICAL CMU REINFORCING, CONTINUOUS FULL HEIGHT, TYPICAL @ ALL WALLS (UNLESS NOTED OTHERWISE).
14. ALL BOND BEAMS AND REINFORCING TO BE CONTINUOUS AROUND CORNERS AND FILLED WITH 3000 P.S.I. GROUT.
15. CONTROL JOINTS IN BRICK VENEER ARE INTENDED TO RELIEVE CONCENTRATED STRESSES BUT NO ASSURANCE IS OFFERED BY THE ENGINEER THAT RANDOM SHRINKAGE CRACKING WILL NOT OCCUR. (SEE ARCHITECTURAL DWG'S FOR LOCATIONS).
16. ALL MASONRY TO BE ADEQUATELY SHORED BY G.C. TO ASSURE NO MOVEMENT UNTIL MORTAR HAS ATTAINED FULL DESIGN STRENGTH.
17. ALL LINTELS SHALL BE SHORED FOR MINIMAL DEFLECTION DURING CONSTRUCTION AND UNTIL MORTAR HAS ATTAINED FULL DESIGN STRENGTH. ASSURANCE FOR PROPER FIT OF OTHER TRADES BELOW LINTEL MUST BE ACHIEVED.
18. SEE SPECIFICATIONS FOR REQUIRED VENEER TIES. PROVIDE HOHMANN & BARNARD DW-10-X ANCHOR WITH BOX TIE W/DRIP WIRE TIES AT STUD/VENEER SYSTEMS. PROVIDE HOHMANN & BARNARD LOX-ALL ADJUSTABLE EYE-WIRE LADDER TYPE #270 WITH RECTANGULAR ADJUSTABLE WALL TIES AT CMU/VENEER SYSTEMS. TIE SPACING SHALL BE 16"O.C. HORIZONTAL AND VERTICAL.
18. ALL BEAMS BEARING ON MASONRY SHALL HAVE A MINIMUM OF 6" BEARING ON WALLS PERPENDICULAR TO THE BEAM SPAN AND A MINIMUM OF THE MAXIMUM BEARING PLATE LENGTH FOR BEARING ON WALLS PARALLEL TO THE BEAM SPAN.

MASONRY FIELD QUALITY CONTROL

THE OWNER WILL EMPLOY AND PAY A QUALIFIED INDEPENDENT TESTING AGENCY TO PERFORM THE FOLLOWING TESTS DURING CONSTRUCTION FOR EACH 5,000 SQ.FT. OF WALL AREA OR PORTION THEREOF AS DIRECTED BY THE ARCHITECT. THE CHOSEN TESTING AGENCY SHALL PROVIDE PROOF OF ACCREDITATION INDICATING CONFORMANCE WITH ASTM C1093 AND INDICATE PERSONNEL, INCLUDING QUALIFICATIONS, WHO SHALL BE PERFORMING PROCEDURES.

MORTAR AND GROUT: COMPOSITION AND PROPERTIES IN ACCORDANCE WITH ASTM.

PRISM TEST METHOD : FOR EACH TYPE OF WALL CONSTRUCTION INDICATED, MASONRY PRISMS WILL BE TESTED PER ASTM E 447, METHOD B. MINIMUM OF ONE SET. ADDITIONAL AS DIRECTED BY THE ARCHITECT.

PREPARE ONE SET OF PRISMS FOR TESTING AT 7 DAYS AND ONE SET FOR TESTING AT 28 DAYS.

DAILY MASONRY INSPECTION: ALL REINFORCED MASONRY SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY ENGAGED BY THE OWNER, OR BY THE CLERK OF THE WORKS, DAILY, IN ACCORDANCE WITH NCMA PUBLICATION TR 156.

INSPECTION / REPORT SHALL BE DOCUMENTED DAILY AND SUBMITTED AT WEEKLY JOB MEETINGS. CONDITIONS NOT IN CONFORMANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS ARE TO BE IMMEDIATELY REPORTED TO THE CONTRACTOR AND ARCHITECT/ENGINEER (A/SER).

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL (SS) TO BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS – ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN", LATEST EDITION.
2. STRUCTURAL STEEL (SS) TO BE ASTM A992 MILL CERTIFIED GRADE 50 (FY = 50,000 P.S.I.).
   TUBE STEEL COLUMNS SHALL BE ASTM A500, GRADE B (FY = 46,000 P.S.I.)
   ROUND COLUMN SECTIONS SHALL BE ASTM A501 (FY = 36,000 P.S.I.) AND ARE DESIGNED BY INSIDE DIAMETER AND ARE TO BE UNFILLED STEEL PIPE.
3. ALL FIELD CONNECTIONS TO BE EITHER BOLTED WITH 3/4" A-325 BOLTS OR WELDED WITH E-70XX ELECTRODES UNLESS NOTED.
4. SUBMIT CONNECTION DETAILS FOR REVIEW PRIOR TO SHOP DRAWING SUBMITTALS. FAILURE TO DO SO MAY RESULT IN REJECTED-NOT APPROVED SUBMITTALS.
5. ALL SS TO RECEIVE ONE COAT OF SHOP PRIMER AND TOUCH-UP IN THE FIELD BY G.C..
6. ALL WELDING TO BE DONE IN ACCORDANCE WITH AMERICAN WELDING SOCIETY BY CERTIFIED WELDERS.
7. ALL SURFACES TO BE FIELD WELDED ARE TO BE FREE OF PAINT, RUST, ETC....
8. ALL COLUMN BASE PLATES TO BE SET ON A 1/4" LEVELING PLATE ON TOP OF 3/4" NON-SHRINK GROUT AND ANCHORED TO FOOTINGS/PIERS WITH ASTM A307 3/4" DIAMETER BOLTS. A.B. LENGTH TO PROVIDE 8"MIN. EMBED. PLUS 3"MIN. HOOK AND SUFFICIENT PROJECTION ABOVE CONCRETE (4"MIN.), TO PROVIDE 1-1/2" MIN. EXTENSION ABOVE BASE PLATE TOP. ALL ANCHOR BOLTS TO BE SET BY TEMPLATE TO ENSURE PROPER POSITIONING.
9. PROVIDE WEB STIFFENERS AT ALL BEAMS FRAMING OVER COLUMNS, BETWEEN COLUMNS OR SUPPORTING COLUMNS.
10. PROVIDE ADDITIONAL DECK SUPPORT WHERE ANY CONNECTION PREVENTS THE DECK FROM BEING ADEQUATELY SUPPORTED BY FRAMING MEMBERS.
11. ALL COLUMNS AND BEAMS EMBEDDED IN CMU WALLS SHALL HAVE MASONRY TIES/ANCHORS @ 16" O.C. EACH SIDE IN DIRECTION OF WALL (UNLESS NOTED).
12. NON-COMPOSITE BEAM CONNECTIONS TO BE SIZED FOR AN END REACTION NOT LESS THAN 1/2 THE TOTAL UNIFORM LOAD CAPACITY OF BEAM FOR A GIVEN SHAPE, SPAN AND GRADE AS SHOWN WITHIN PART 2 – "ALLOWABLE LOADS ON BEAMS" TABLES OF THE 9TH ED. AISC MANUAL, UNLESS OTHERWISE SHOWN.

MISC. STEEL

1. ALL MISC. STEEL SHALL BE ASSUMED TO BE ATTACHED BACK TO THE SUPER-STRUCTURE UNLESS SHOWN OTHERWISE.
2. PROVIDE ALL MISCELLANEOUS STEEL FOR OPENINGS, ATTACHMENT OF THE WORK OF OTHER TRADES, ETC. AS REQUIRED BY THE INTENT OF ALL CONTRACT DOCUMENTS.
3. ALL LINTELS SHALL BE SHORED FOR MINIMAL DEFLECTION DURING CONSTRUCTION AND UNTIL MORTAR HAS ATTAINED FULL DESIGN STRENGTH. ASSURANCE FOR PROPER FIT OF OTHER TRADES BELOW LINTEL MUST BE ACHIEVED.
4. SUPPORT MECHANICAL UNITS, SPRINKLER PIPING AND WATER SUPPLY PIPING, MISC EQUIPMENT, ETC..., FROM FRAMING, OR UNI-STRUT ATTACHED BACK TO FRAMING. DO NOT ATTACH HANGERS TO FLOOR OR ROOF DECK. COORDINATE CONNECTION WITH ENGINEER PRIOR TO INSTALLATION.

TIMBER FRAMING

1. ALL FRAMING SHALL BE IN ACCORDANCE WITH ALL AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (A.I.T.C.) SPECIFICATIONS, LATEST EDITION.
2. FRAMING MEMBERS SHALL CONFORM TO NFPA "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION AND MEET OR EXCEED THE FOLLOWING SPECIES PROPERTIES UNLESS NOTED OTHERWISE:

   ALL MEMBERS UNLESS NOTED:
      SPRUCE-PINE-FIR #2 OR BETTER
         (Fb=875 PSI (MIN), E=1400 KSI (MIN))

   MANUFACTURED LUMBER:
   LAMINATED VENEER LUMBER (LVL) MEMBERS ARE TO BE IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.
      MICRO=LAM BY TRUSS JOIST CORP.
         (Fb=2600 PSI (MIN), E=1900 KSI (MIN))
      PARALLAM PSL BY TRUSS JOIST CORP.
         (Fb=2900 PSI (MIN), E=2000 KSI (MIN))
      TIMBER STRAND LSL BY TRUSS JOIST CORP.
         (Fb=1700 PSI (MIN), E=1300 KSI (MIN))
         (Fb=2250 PSI (MIN), E=1500 KSI (MIN))
      TJI JOISTS BY TRUSS JOIST CORP.
3. ALL FRAMING MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SYP #2 OR BETTER.
4. CONNECTION HARDWARE AND IT'S INSTALLATION SHALL BE IN ACCORDANCE WITH A.I.T.C. SPECIFICATIONS AND MANUFACTURERS RECOMMENDATIONS.

NAILING SCHEDULE

THE FOLLOWING ARE MINIMUM TYPICAL UNLESS OTHERWISE NOTED.

CONNECTION	NAILING
JOIST TO SILL OR GIRDER, TOENAIL	3 – 8D
SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16D @ 16" O.C.
TOP PLATE TO STUD, END NAIL	2 – 16D
STUD TO SOLE PLATE	4 – 8D TOE NAIL OR 2 – 16D END NAIL
DOUBLE STUDS, FACE NAIL	16D @ 24" O.C.
DOUBLE TOP PLATES, FACE NAIL	16D @ 16" O.C.
TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2 – 16D
CONTINUOUS HEADER TO STUD, TOE NAIL	16D @ 16" O.C. ALONG EACH EDGE
BUILT-UP CORNER STUDS	16D @ 24" O.C.

PLYWOOD SHEATHING

1. ALL PLYWOOD TO BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH ALL APA THE ENGINEERED WOOD ASSOCIATION SPECIFICATIONS, LATEST EDITION.
2. ALL SHEATHING TO BE INSTALLED WITH THE LONG DIMENSION OF THE PANEL ACROSS SUPPORTS AND WITH THE PANEL CONTINUOUS OVER TWO OR MORE SPANS.
3. FLOOR SHEATHING (SUBFLOORING) TO BE 3/4", APA RATED STURD-I-FLOOR 20" OC MIN. SPAN RATING, T&G, EXPOSURE 1 (UNLESS NOTED). SHEATHING TO BE GLUED WITH ADHESIVES MEETING APA SPEC. AFG-01 AND NAILED WITH (8d COMMON) OR (6D RING- OR SCREW-SHANK) NAILS AT 12" O.C. ALONG PANEL EDGES AND AT INTERMEDIATE SUPPORTS.
4. ROOF SHEATHING TO BE 5/8", APA RATED SHEATHING, 40/20 MIN. SPAN RATING, EXPOSURE 1 (UNLESS NOTED). SHEATHING TO BE NAILED WITH 8D RING- OR SCREW-SHANK NAILS AT 6" O.C. ALONG SUPPORTED PANEL EDGES AND 12" O.C. ALONG INTERMEDIATE SUPPORTS. PROVIDE FIRE RETARDANT TREATED PLYWOOD AS NOTED BY ARCHITECT.
5. EXTERIOR WALL SHEATHING TO BE 1/2" APA RATED SHEATHING, EXPOSURE 1 UNLESS NOTED. SHEATHING TO BE NAILED WITH 6D NAILS AT 6" O.C. ALONG PANEL EDGES (PROVIDE BLOCKING AS REQUIRED) AND 12" O.C. ALONG INTERMEDIATE SUPPORTS.

THIS LIST IS TO BE USED IN CONJUNCTION WITH THE CONTRACT SPECIFICATIONS AND IS NOT TO BE CONSTRUED AS LIMITING THE GENERAL CONTRACTOR'S RESPONSIBILITIES FOR ANY ITEMS CONTRACTUALLY OR TRADITIONALLY REQUIRED.

DESIGN LOADS

(IN ACCORDANCE W/ 1996 BOCA)

FLOOR LIVE LOADS (UNIFORMLY DISTRIBUTED):
4" SLAB ON GRADE .... 100 PSF
6" SLAB ON GRADE .... 200 PSF
SUPPORTED FLOOR LIVE LOADS:
ASSEMBLY (FIXED SEATS) .... 60 PSF
ASSEMBLY (MOVABLE SEATS) ..... 100 PSF
PROJECTION ROOM .... 50 PSF
LOBBIES .... 100 PSF
CORRIDORS .... 80 PSF
CLASSROOMS & MISC. ROOMS ..... 40 PSF
OFFICES .... 50 PSF

STAIRS & LANDINGS .... 100 PSF

CONCENTRATED LOADS:
FLOOR .... 2000 POUNDS
STAIRS .... 300 POUNDS
(COMPLY WITH BOCA LOADING REQUIREMENTS FOR:
STAIRS, STAIR RAILINGS, GUARDS AND HANDRAILS)

ROOF SNOW LOADS
GROUND SNOW LOAD (Pg) .... 60 PSF
EXPOSURE FACTOR (Ce) 0.7
IMPORTANCE FACTOR (I) .... 1.0 (ELEVATOR LOBBY)
RAIN ON SNOW SURCHARGE .... 3 PSF
THERMAL FACTOR (Ct) 1.0
ROOF SLOPE FACTOR (Cs) 1.0
FLAT ROOF SNOW LOAD (Pf):
FLAT ROOFS .... 45 PSF (PLUS SNOW DRIFTING/SLIDING)

ROOF DEAD LOADS
BALLASTED (AT FLAT ROOFS) .... 12 PSF (MAX.)

WIND LOAD (STRUCTURAL SYSTEM DESIGN ONLY – NOT FOR COMPONENTS, CLADDING NOR CURTAIN WALL SYSTEMS)
BASIC WIND SPEED .... 80 MPH
WIND LOAD IMPORTANCE FACTOR (I) ... 1.21
WIND EXPOSURE CATEGORY .... EXPOSURE B
WIND DESIGN PRESSURE (P)........... 15 PSF
WIND UPLIFT .... 15 PSF

EARTHQUAKE DESIGN DATA (NEW CONSTRUCTION)
PEAK VELOCITY (Av) .... .13
PEAK ACCELERATION (Aa) .... .13
SEISMIC HAZARD EXPOSURE GROUP ..... GROUP I
SEISMIC PERFORMANCE CATEGORY C
SOIL-PROFILE TYPE/COEFFICIENT ..... S1 / 1.0
BASIC STRUCTURAL SYSTEM .... TYPE 5
(WOOD SHEATHED SHEAR WALLS / REINF. MASONRY SHEAR WALLS)
RESPONSE MODIFICATION FACTOR (R) .. 7
DEFLECT. AMPLIF. FACTOR (Cd) 4.5
ANALYSIS PROCEDURE .... EQUIV. LATERAL FORCE PROCEDURE

EARTHQUAKE DESIGN DATA (EXISTING CONSTRUCTION)
NOT APPLICABLE – UNCHANGED

REV. NO.	REVISIONS	DATE

PROJECT:
TOWN OF DERRY
ADAMS MEMORIAL BUILDING RENOVATIONS
DERRY NEW HAMPSHIRE

SUBJECT:
GENERAL NOTES & SCHEDULES
SCALE: AS NOTED DATE: 06/09/00

UDELSMAN ASSOCIATES
ARCHITECTURE PLANNING DESIGN
161 FEDERAL HILL ROAD
HOLLIS, NEW HAMPSHIRE 03049
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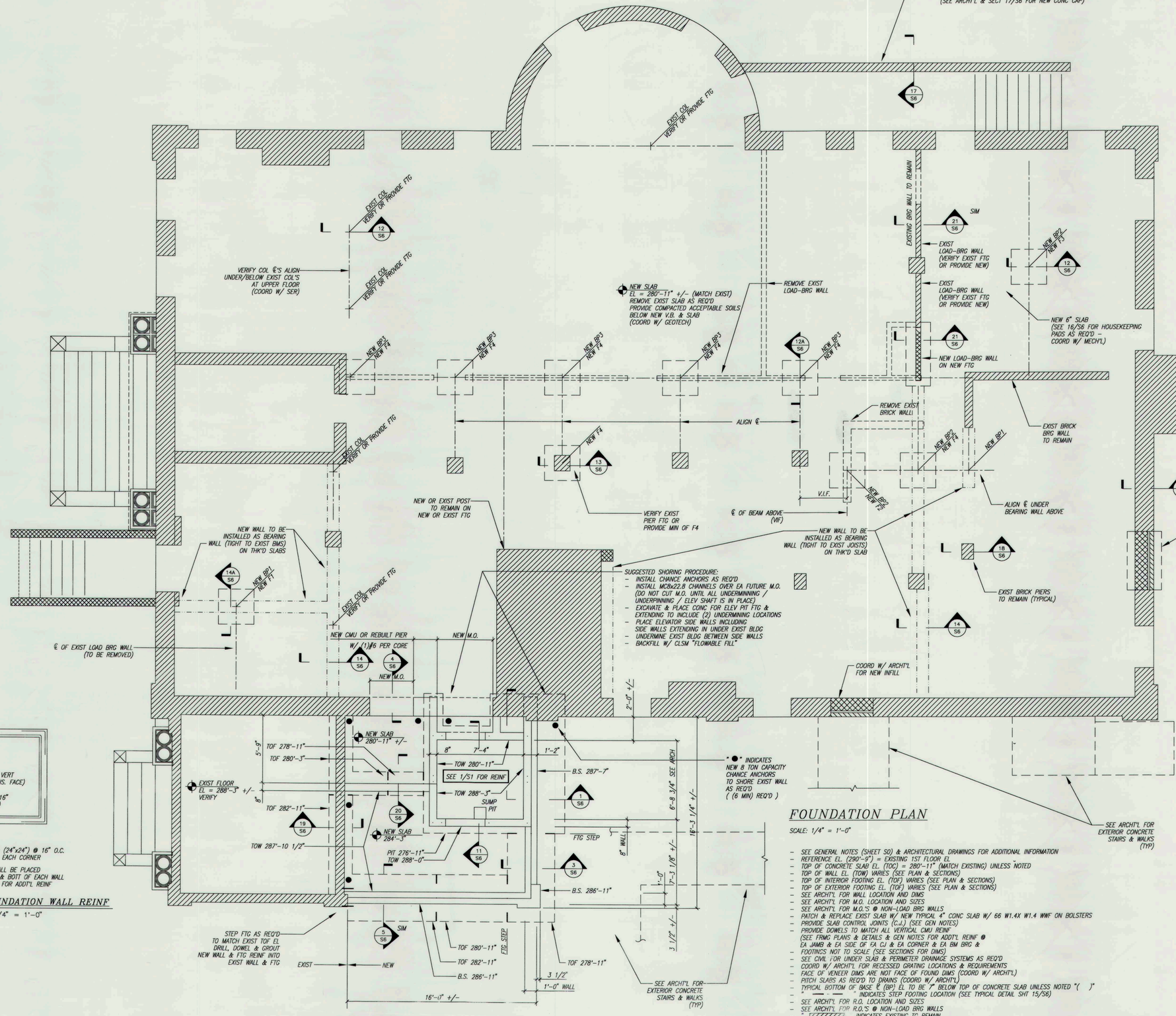
STEFFENSEN ENGINEERING ASSOC., INC.
CONSULTING
STRUCTURAL ENGINEERS
31 CALEF ROAD
AUBURN, NH 03032
(603) 483-1300
(FAX) 483-0650

PROJECT NO.:
99-064

SHEET NO.:
S0



1 / S1 ELEVATOR FOUNDATION WALL REINF

3/4" = 1'-0"

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

- SEE GENERAL NOTES (SHEET S0) & ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION
- REFERENCE EL. (290'-9") = EXISTING 1ST FLOOR EL
- TOP OF CONCRETE SLAB EL. (TOC) = 280'-11" (MATCH EXISTING) UNLESS NOTED
- TOP OF WALL EL. (TOW) VARIES (SEE PLAN & SECTIONS)
- TOP OF INTERIOR FOOTING EL. (TOF) VARIES (SEE PLAN & SECTIONS)
- TOP OF EXTERIOR FOOTING EL. (TOF) VARIES (SEE PLAN & SECTIONS)
- SEE ARCHT'L FOR WALL LOCATION AND DIMS
- SEE ARCHT'L FOR M.O. LOCATION AND SIZES
- SEE ARCHT'L FOR M.O.'S @ NON-LOAD BRG WALLS
- PATCH & REPLACE EXIST SLAB W/ NEW TYPICAL 4" CONC SLAB W/ 6G W1.4X W1.4 WWF ON BOLSTERS
- PROVIDE SLAB CONTROL JOINTS (C.J.) (SEE GEN NOTES)
- PROVIDE DOWELS TO MATCH ALL VERTICAL CMU REINF (SEE FRMG PLANS & DETAILS & GEN NOTES FOR ADDT'L REINF @ EA JAMB & EA SIDE OF EA CJ & EA CORNER & EA BM BRG &
- FOOTINGS NOT TO SCALE (SEE SECTIONS FOR DIMS)
- SEE CIVIL FOR UNDER SLAB & PERIMETER DRAINAGE SYSTEMS AS REQ'D
- COORD W/ ARCHT'L FOR RECESSED GRATING LOCATIONS & REQUIREMENTS
- FACE OF VENEER DIMS ARE NOT FACE OF FOUND DIMS (COORD W/ ARCHT'L)
- PITCH SLABS AS REQ'D TO DRAINS (COORD W/ ARCHT'L)
- TYPICAL BOTTOM OF BASE ℄ (BP) EL TO BE 1' BELOW TOP OF CONCRETE SLAB UNLESS NOTED "( )"
- " ——— INDICATES STEP FOOTING LOCATION (SEE TYPICAL DETAIL SHT 15/S6)
- SEE ARCHT'L FOR R.O. LOCATION AND SIZES
- SEE ARCHT'L FOR R.O.'S @ NON-LOAD BRG WALLS
- " ▨ INDICATES EXISTING TO REMAIN
- SEE FRMG PLANS FOR ALL BEAM & JOIST POCKETS & BRG PL'S
- B.S. INDICATES BRICK SHELF

REV. NO.	REVISIONS	DATE

TOWN OF DERRY
MEMORIAL BUILDING RENOVATIONS
NEW HAMPSHIRE

PROJECT: ADAMS MEMORIAL BUILDING, DERRY

DATE: 06/09/00

SUBJECT: FOUNDATION PLAN

SCALE: AS NOTED

UDELSMAN ASSOCIATES
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161 FEDERAL HILL ROAD
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181 SCALEHILL ROAD
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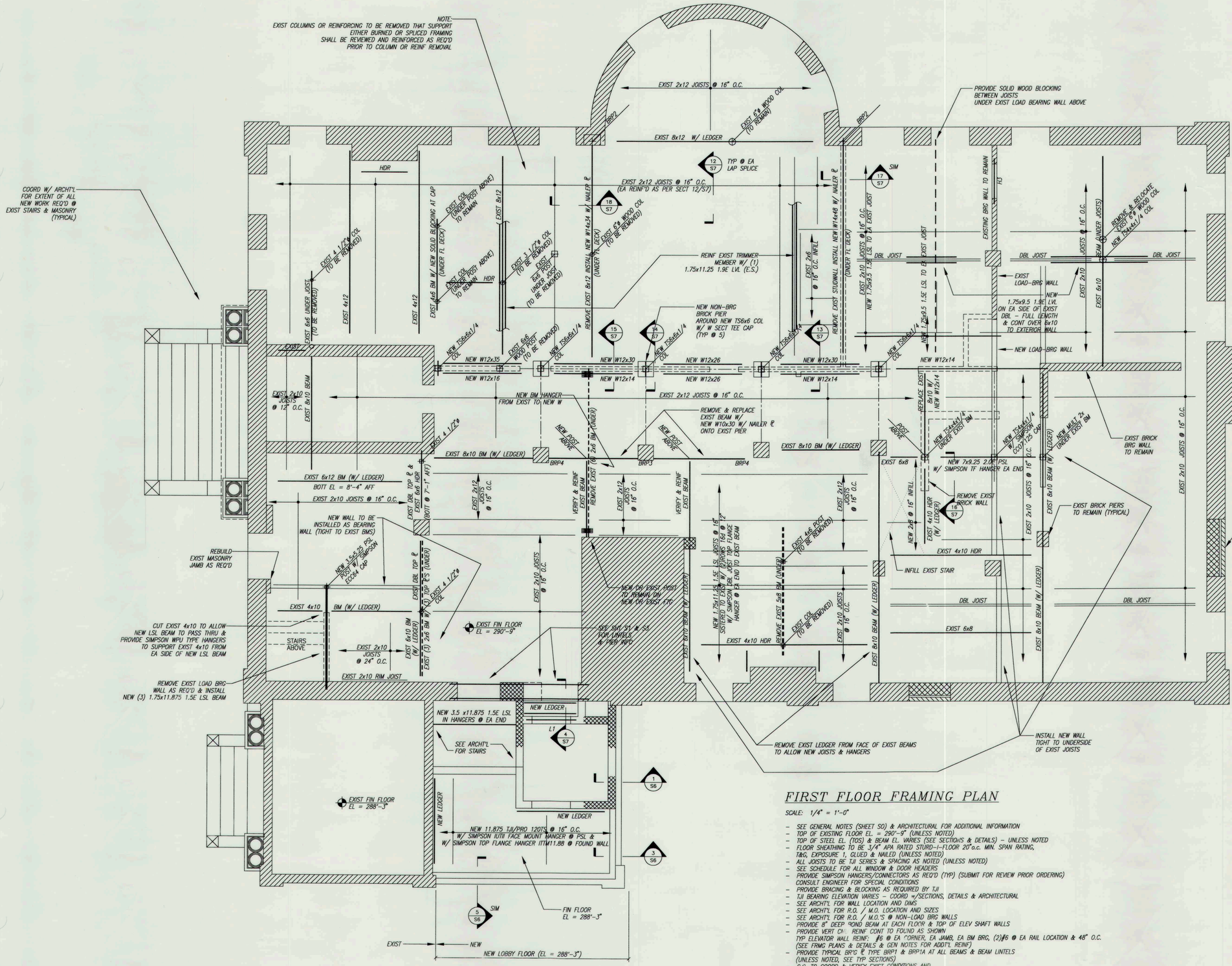
(603) 483-1300
(FAX) 483-0650

PROJECT NO.: 99-064

SHEET NO.: S1



FIRST FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

- SEE GENERAL NOTES (SHEET S0) & ARCHITECTURAL FOR ADDITIONAL INFORMATION
- TOP OF EXISTING FLOOR EL. = 290'-9" (UNLESS NOTED)
- TOP OF STEEL EL. (TOS) & BEAM EL. VARIES (SEE SECTIONS & DETAILS) - UNLESS NOTED
- FLOOR SHEATHING TO BE 3/4" APA RATED STURD-I-FLOOR 20"o.c. MIN. SPAN RATING, T&G, EXPOSURE 1, GLUED & NAILED (UNLESS NOTED)
- ALL JOISTS TO BE TJI SERIES & SPACING AS NOTED (UNLESS NOTED)
- SEE SCHEDULE FOR ALL WINDOW & DOOR HEADERS
- PROVIDE SIMPSON HANGERS/CONNECTORS AS REQ'D (TYP) (SUBMIT FOR REVIEW PRIOR ORDERING) CONSULT ENGINEER FOR SPECIAL CONDITIONS
- PROVIDE BRACING & BLOCKING AS REQUIRED BY TJI
- TJI BEARING ELEVATION VARIES - COORD w/SECTIONS, DETAILS & ARCHITECTURAL
- SEE ARCHT'L FOR WALL LOCATION AND DIMS
- SEE ARCHT'L FOR R.O. / M.O. LOCATION AND SIZES
- SEE ARCHT'L FOR R.O. / M.O.'S @ NON-LOAD BRG WALLS
- PROVIDE 8" DEEP ®OND BEAM AT EACH FLOOR & TOP OF ELEV SHAFT WALLS
- PROVIDE VERT CH. REINF CONT TO FOUND AS SHOWN TYP ELEVATOR WALL REINF: #6 @ EA CORNER, EA JAMB, EA BM BRG, (2)#6 @ EA RAIL LOCATION & 48" O.C. (SEE FRMG PLANS & DETAILS & GEN NOTES FOR ADD'L REINF)
- PROVIDE TYPICAL BR'G ℄ TYPE BRP1 & BRP1A AT ALL BEAMS & BEAM LINTELS (UNLESS NOTED, SEE TYP SECTIONS)
- G.C. TO COORD & VERIFY EXIST CONDITIONS AND DIMENSIONS. INFORM A/E IF OTHER THAN AS SHOWN/NOTED.

NORTH

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REV. NO.	REVISIONS	DATE

PROJECT: ADAMS MEMORIAL BUILDING RENOVATIONS, DERRY — TOWN OF DERRY, NEW HAMPSHIRE

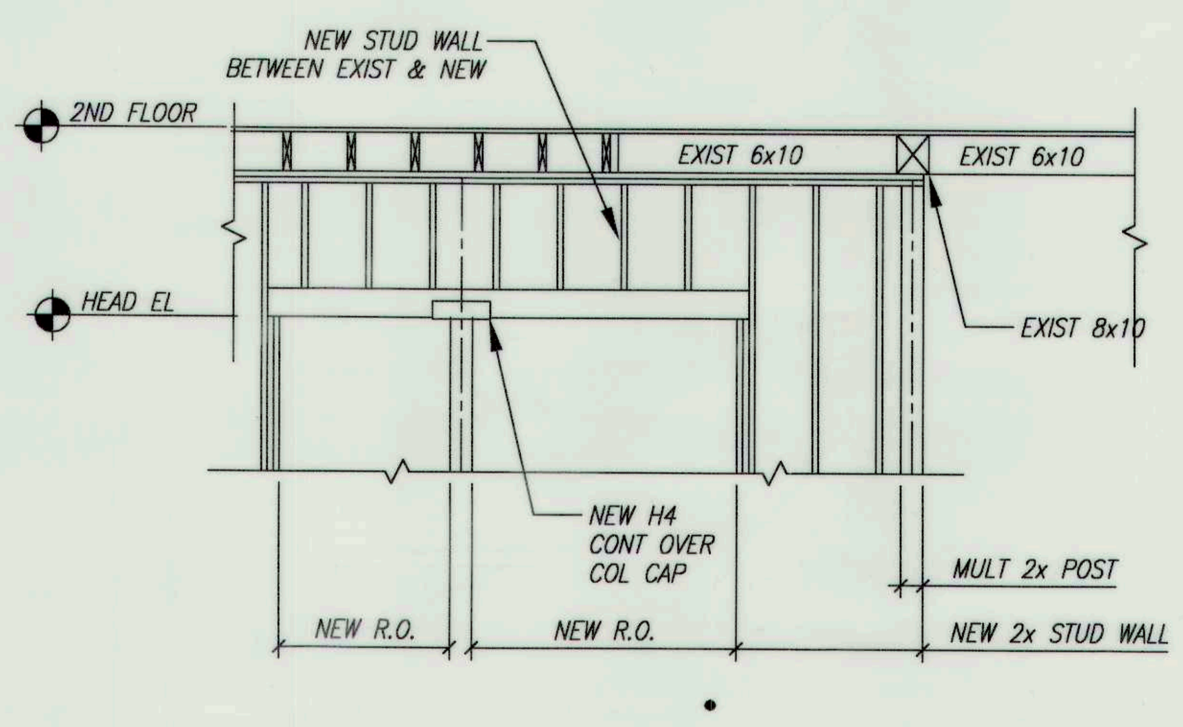
SUBJECT: FIRST FLOOR FRAMING PLAN — DATE: 06/09/00

SCALE: AS NOTED

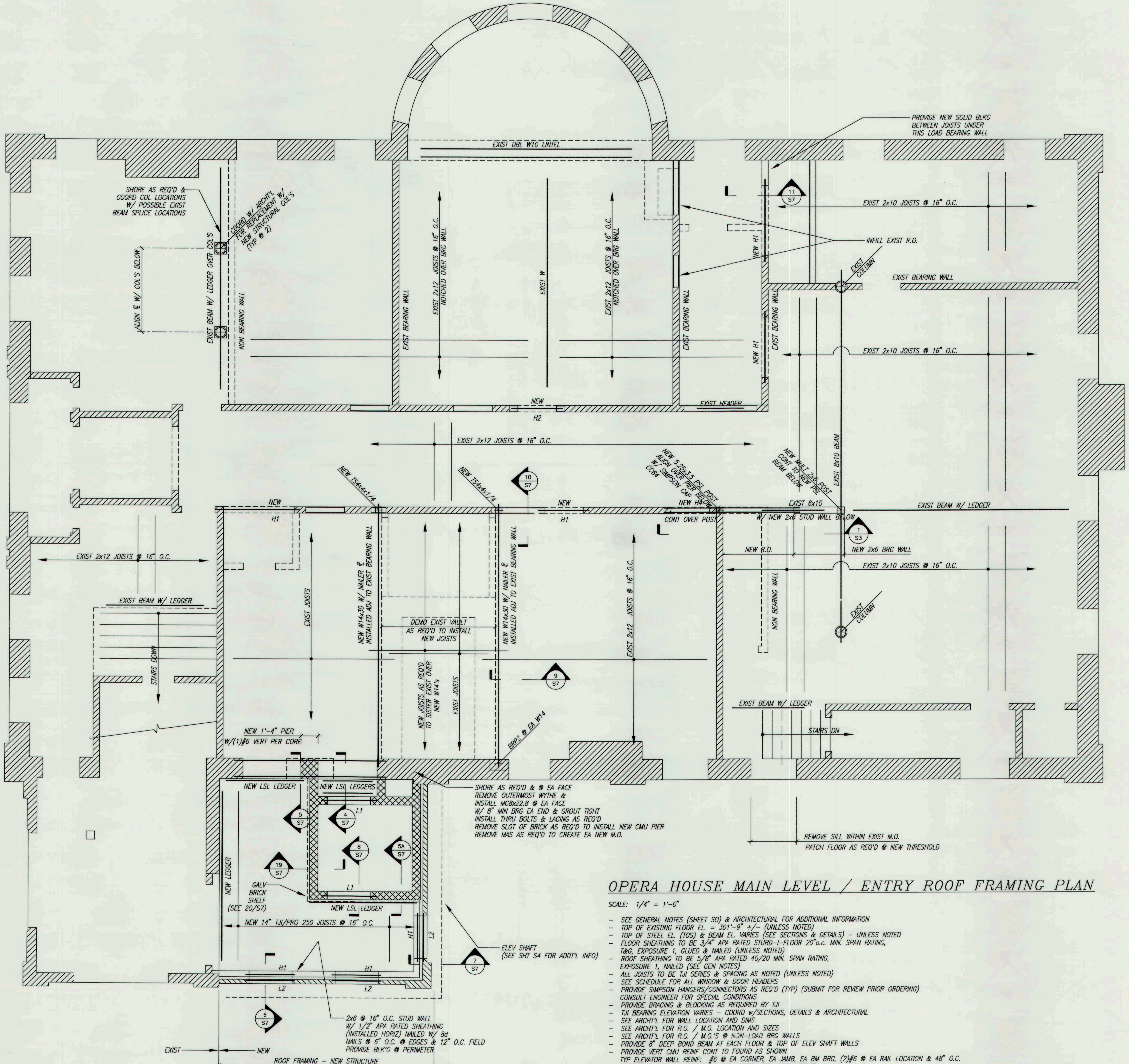
UDELSMAN ASSOCIATES
ARCHITECTURE PLANNING DESIGN
161 FEDERAL HILL ROAD
HOLLIS, NEW HAMPSHIRE 03049
603 465 6960 FAX 465 6961

PROJECT NO.: 99-064

SHEET NO.: S2



1 S3 TYPICAL SECTION
3/4" = 1'-0"



OPERA HOUSE MAIN LEVEL / ENTRY ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

- SEE GENERAL NOTES (SHEET S0) & ARCHITECTURAL FOR ADDITIONAL INFORMATION
- TOP OF EXISTING FLOOR EL. = 301'-9" +/- (UNLESS NOTED)
- TOP OF STEEL EL. (TOS) & BEAM EL. VARIES (SEE SECTIONS & DETAILS) - UNLESS NOTED
- FLOOR SHEATHING TO BE 3/4" APA RATED STURD-I-FLOOR 20"o.c. MIN. SPAN RATING, T&G, EXPOSURE 1, GLUED & NAILED (UNLESS NOTED)
- ROOF SHEATHING TO BE 5/8" APA RATED 40/20 MIN. SPAN RATING, EXPOSURE 1, NAILED (SEE GEN NOTES)
- ALL JOISTS TO BE TJI SERIES & SPACING AS NOTED (UNLESS NOTED)
- SEE SCHEDULE FOR ALL WINDOW & DOOR HEADERS
- PROVIDE SIMPSON HANGERS/CONNECTORS AS REQ'D (TYP) (SUBMIT FOR REVIEW PRIOR ORDERING) CONSULT ENGINEER FOR SPECIAL CONDITIONS
- PROVIDE BRACING & BLOCKING AS REQUIRED BY TJI
- TJI BEARING ELEVATION VARIES - COORD w/SECTIONS, DETAILS & ARCHITECTURAL
- SEE ARCHT'L FOR WALL LOCATION AND DIMS
- SEE ARCHT'L FOR R.O. / M.O. LOCATION AND SIZES
- SEE ARCHT'L FOR R.O. / M.O.'S @ NON-LOAD BRG WALLS
- PROVIDE 8" DEEP BOND BEAM AT EACH FLOOR & TOP OF ELEV SHAFT WALLS
- PROVIDE VERT CMU REINF CONT TO FOUND AS SHOWN
  TYP ELEVATOR WALL REINF: #6 @ EA CORNER, EA JAMB, EA BM BRG, (2)#6 @ EA RAIL LOCATION & 48" O.C.
  (SEE FRMG PLANS & DETAILS & GEN NOTES FOR ADDT'L REINF)
- PROVIDE TYPICAL BR'G ℙ TYPE BRP1 & BRP1A AT ALL BEAMS & BEAM LINTELS (UNLESS NOTED, SEE TYP SECTIONS)
- G.C. TO COORD & VERIFY EXIST CONDITIONS AND DIMENSIONS. INFORM A/E IF OTHER THAN AS SHOWN/NOTED.

NORTH

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REV. NO.	REVISIONS	DATE

PROJECT: TOWN OF DERRY
ADAMS MEMORIAL BUILDING RENOVATIONS
DERRY NEW HAMPSHIRE

SUBJECT: OPERA HOUSE MAIN LEVEL FLOOR FRAMING
ENTRY ROOF FRAMING PLAN

SCALE: AS NOTED DATE: 06/09/00

UDELSMAN ASSOCIATES
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HOLLIS, NEW HAMPSHIRE 03049
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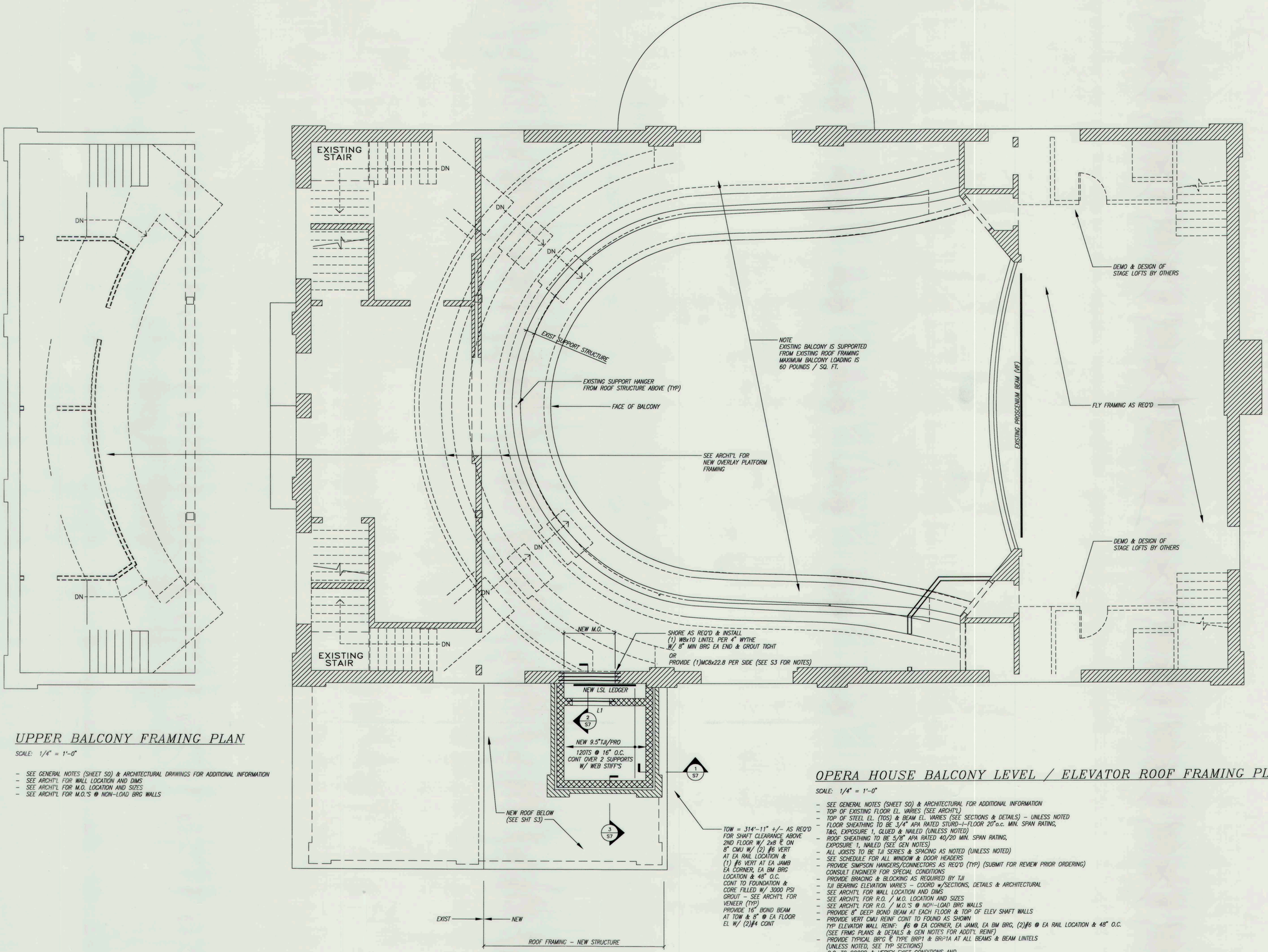
PROJECT NO.: 99-064

SHEET NO.: S3



UPPER BALCONY FRAMING PLAN

SCALE: 1/4" = 1'-0"

- SEE GENERAL NOTES (SHEET S0) & ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION
- SEE ARCHT'L FOR WALL LOCATION AND DIMS
- SEE ARCHT'L FOR M.O. LOCATION AND SIZES
- SEE ARCHT'L FOR M.O.'S @ NON-LOAD BRG WALLS

OPERA HOUSE BALCONY LEVEL / ELEVATOR ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

- SEE GENERAL NOTES (SHEET S0) & ARCHITECTURAL FOR ADDITIONAL INFORMATION
- TOP OF EXISTING FLOOR EL. VARIES (SEE ARCHT'L)
- TOP OF STEEL EL. (TOS) & BEAM EL. VARIES (SEE SECTIONS & DETAILS) - UNLESS NOTED
- FLOOR SHEATHING TO BE 3/4" APA RATED STURD-I-FLOOR 20"o.c. MIN. SPAN RATING, T&G, EXPOSURE 1, GLUED & NAILED (UNLESS NOTED)
- ROOF SHEATHING TO BE 5/8" APA RATED 40/20 MIN. SPAN RATING, EXPOSURE 1, NAILED (SEE GEN NOTES)
- ALL JOISTS TO BE TJI SERIES & SPACING AS NOTED (UNLESS NOTED)
- SEE SCHEDULE FOR ALL WINDOW & DOOR HEADERS
- PROVIDE SIMPSON HANGERS/CONNECTORS AS REQ'D (TYP) (SUBMIT FOR REVIEW PRIOR ORDERING) CONSULT ENGINEER FOR SPECIAL CONDITIONS
- PROVIDE BRACING & BLOCKING AS REQUIRED BY TJI
- TJI BEARING ELEVATION VARIES - COORD w/SECTIONS, DETAILS & ARCHITECTURAL
- SEE ARCHT'L FOR WALL LOCATION AND DIMS
- SEE ARCHT'L FOR R.O. / M.O. LOCATION AND SIZES
- SEE ARCHT'L FOR R.O. / M.O.'S @ NON-LOAD BRG WALLS
- PROVIDE 8" DEEP BOND BEAM AT EACH FLOOR & TOP OF ELEV SHAFT WALLS
- PROVIDE VERT CMU REINF CONT TO FOUND AS SHOWN TYP ELEVATOR WALL REINF: #6 @ EA CORNER, EA JAMB, EA BM BRG, (2)#6 @ EA RAIL LOCATION & 48" O.C. (SEE FRMG PLANS & DETAILS & GEN NOTES FOR ADDT'L REINF)
- PROVIDE TYPICAL BR'G @ TYPE BRP1 & BR@1A AT ALL BEAMS & BEAM LINTELS (UNLESS NOTED, SEE TYP SECTIONS)
- G.C. TO COORD & VERIFY EXIST CONDITIONS AND DIMENSIONS. INFORM A/E IF OTHER THAN AS SHOWN/NOTED.

NORTH

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(603) 483-1300
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REV. NO.	REVISIONS	DATE

PROJECT: TOWN OF DERRY
ADAMS MEMORIAL BUILDING RENOVATIONS
DERRY NEW HAMPSHIRE

SUBJECT: OPERA HOUSE BALCONY LEVEL FLOOR FRAMING
SCALE: AS NOTED ELEVATOR ROOF FRAMING PLAN DATE: 06/09/00

UDELSMAN ASSOCIATES
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161 FEDERAL HILL ROAD
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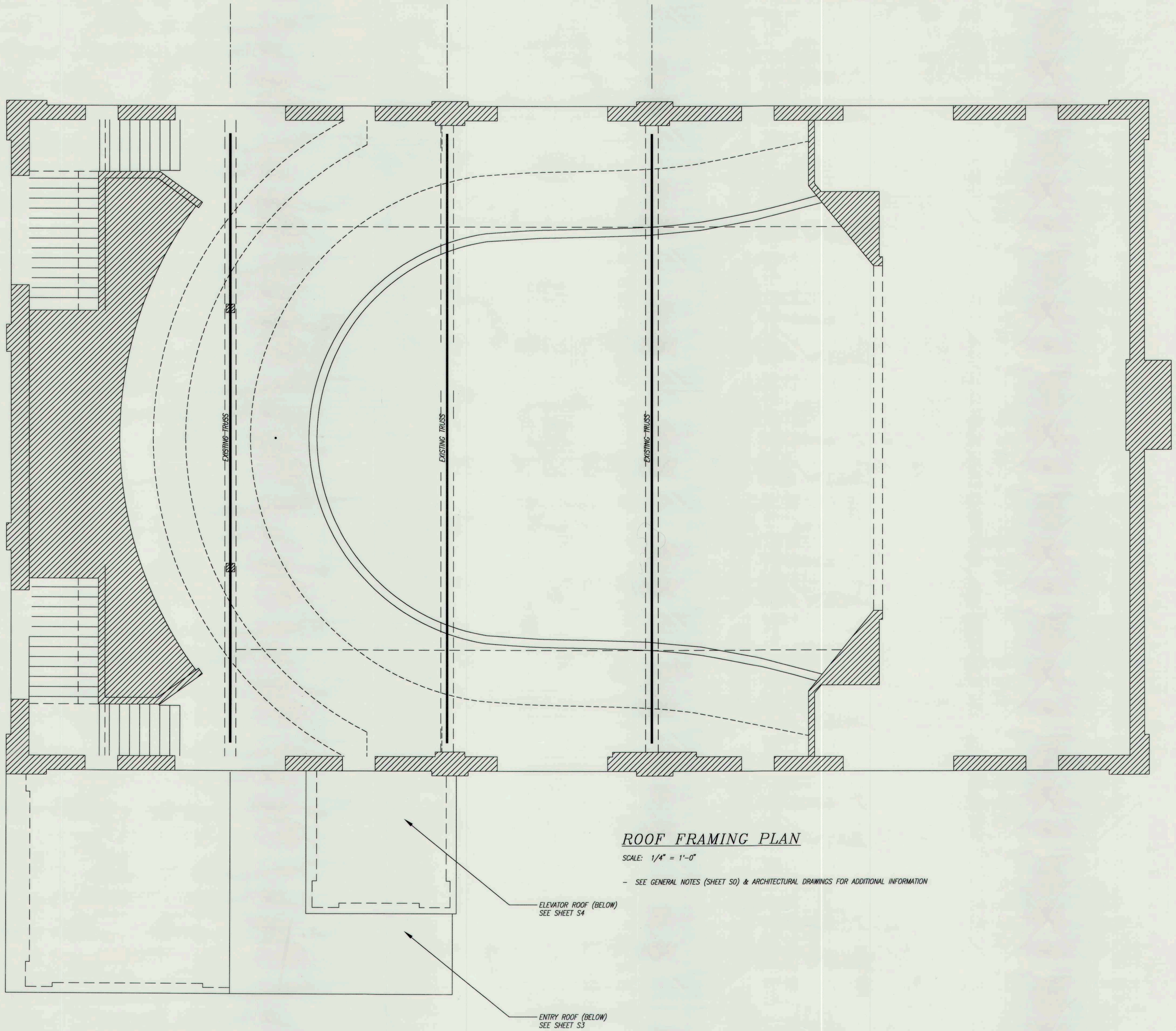
603 465-6960 FAX 465-6961

PROJECT NO.: 99-064

SHEET NO.: S4



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

- SEE GENERAL NOTES (SHEET S0) & ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION

REV. NO.	REVISIONS	DATE

PROJECT:
TOWN OF DERRY
ADAMS MEMORIAL BUILDING RENOVATIONS
DERRY NEW HAMPSHIRE

SUBJECT:
ROOF FRAMING PLAN

SCALE: AS NOTED
DATE: 06/09/00

UDELSMAN ASSOCIATES
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AUBURN, NH 03032
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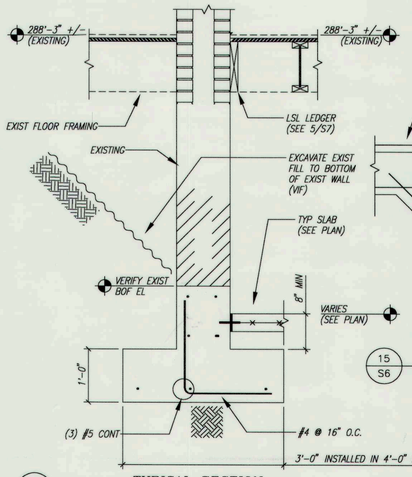
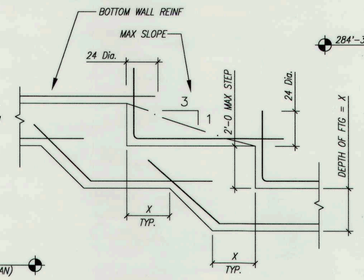
NORTH

PROJECT NO.: 99-064

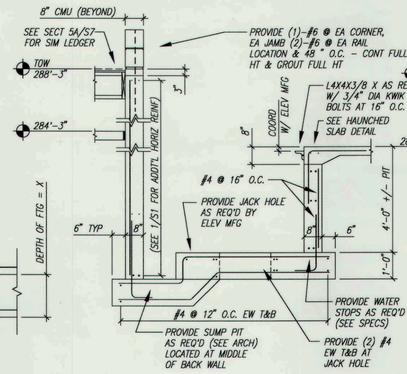
SHEET NO.: S5



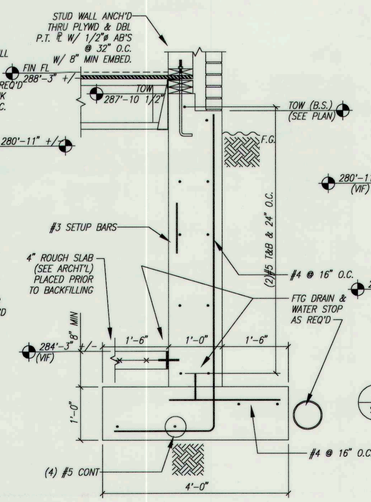
19/S6 TYPICAL SECTION
3/4" = 1'-0"



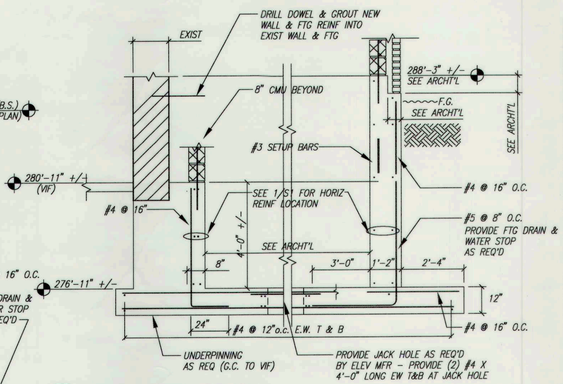
15/S6 STEPPED FOOTING DETAIL
3/4" = 1'-0"



11/S6 SECTION @ ELEVATOR PIT
NTS



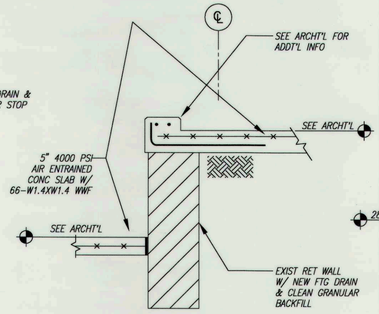
5/S6 TYPICAL SECTION
3/4" = 1'-0"



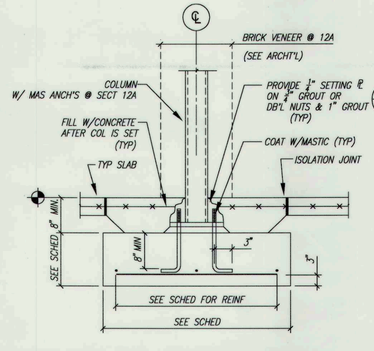
1/S6 SECTION
3/8" = 1'-0"

20/S6 SECTION
3/4" = 1'-0"

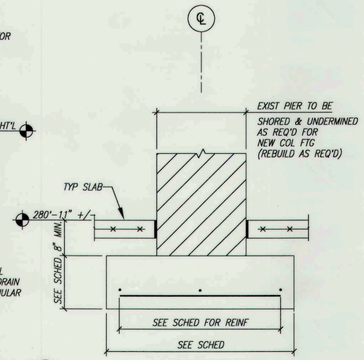
16/S6 HOUSEKEEPING PAD DETAIL
NTS



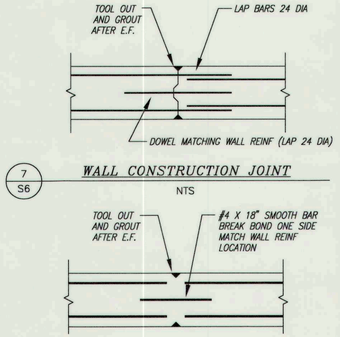
17/S6 EXTERIOR SECTION
3/4" = 1'-0"



12/S6 12A/S6 TYPICAL INTERIOR FOOTING
3/4" = 1'-0"



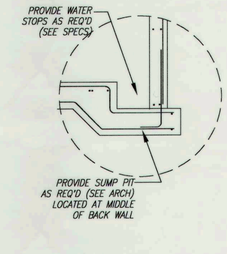
13/S6 SECTION
3/4" = 1'-0"



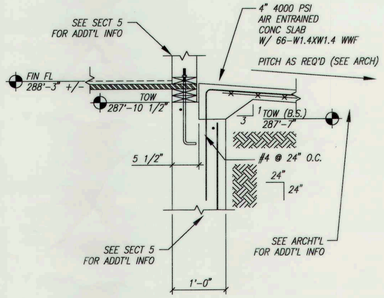
6/S6 6" SLAB CONTROL JOINT
3/4" = 1'-0"

7/S6 WALL CONSTRUCTION JOINT
NTS

8/S6 WALL CONTROL JOINT
NTS



2/S6 ELEVATOR SUMP PIT SECTION
3/4" = 1'-0"



3/S6 TYPICAL ENTRY SECTION
3/4" = 1'-0"

21/S6 TYPICAL SECTION
3/4" = 1'-0"

18/S6 TYPICAL SECTION
3/4" = 1'-0"

14/S6 14A/S6 THICKENED SLAB DETAIL
3/4" = 1'-0"

9/S6 4" SLAB CONSTRUCTION JOINT
NTS

10/S6 4" SLAB CONTROL JOINT
NTS

4/S6 HAUNCHED SLAB DETAIL
3/4" = 1'-0"

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(FAX) 483-0650

REV. NO.	REVISIONS	DATE

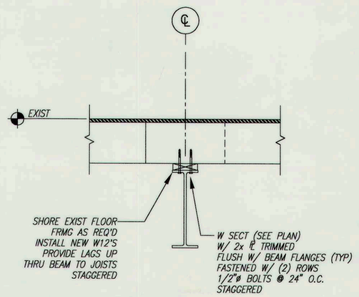
PROJECT: TOWN OF DERRY
ADAMS MEMORIAL BUILDING RENOVATIONS
DERRY NEW HAMPSHIRE

SUBJECT: FOUNDATION SECTIONS & DETAILS
SCALE: AS NOTED
DATE: 06/09/00

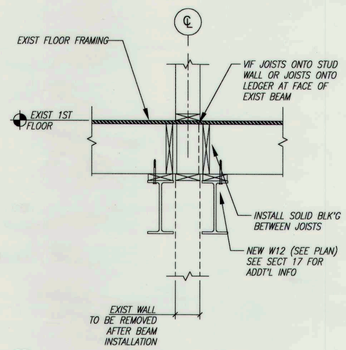
UDELSMAN ASSOCIATES
ARCHITECTURE PLANNING DESIGN
161 FEDERAL HILL ROAD
HOLLIS, NEW HAMPSHIRE 03049
603 465-6960 FAX 465-6961

PROJECT NO.: 99-064

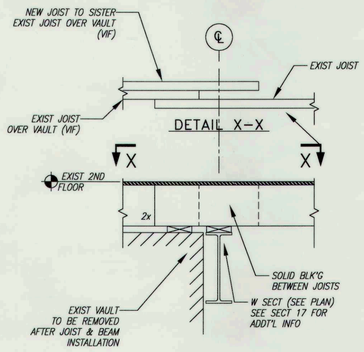
SHEET NO.: S6



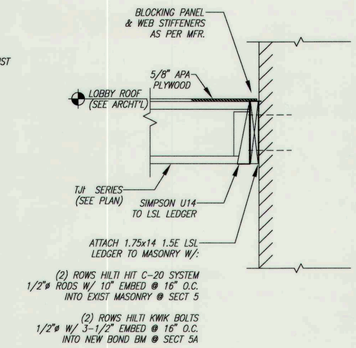
17/S7 BEAM PERPENDICULAR SECTION
3/4" = 1'-0"



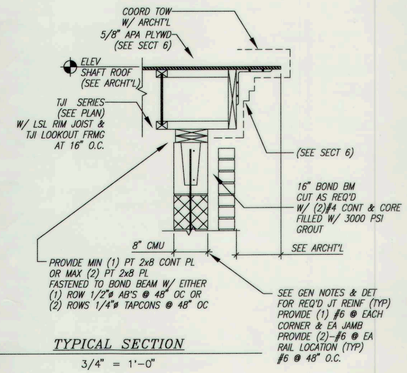
13/S7 TYPICAL SECTION
3/4" = 1'-0"



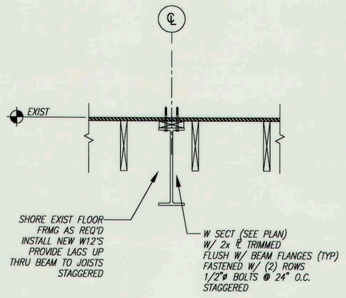
9/S7 TYPICAL SECTION
3/4" = 1'-0"



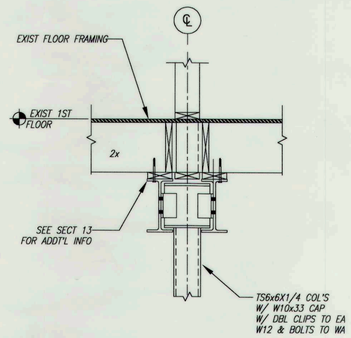
5 S7 5A S7 TYPICAL SECTION
3/4" = 1'-0"



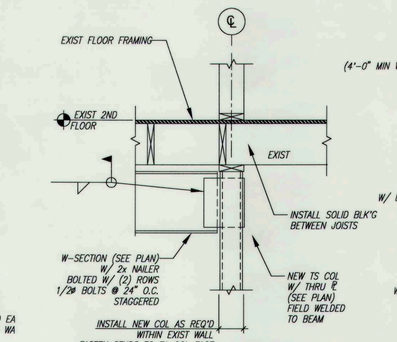
1/S7 TYPICAL SECTION
3/4" = 1'-0"



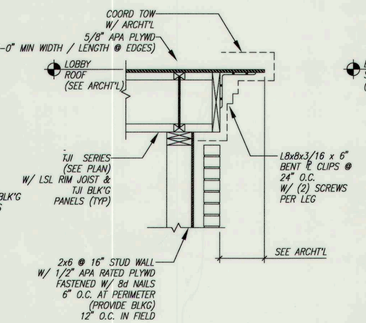
18/S7 BEAM PARALLEL SECTION
3/4" = 1'-0"



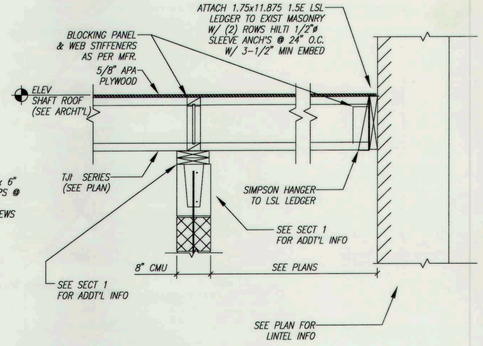
14/S7 TYPICAL SECTION
3/4" = 1'-0"



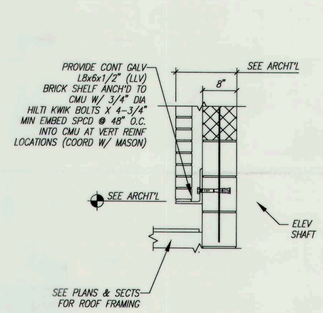
10/S7 TYPICAL SECTION
3/4" = 1'-0"



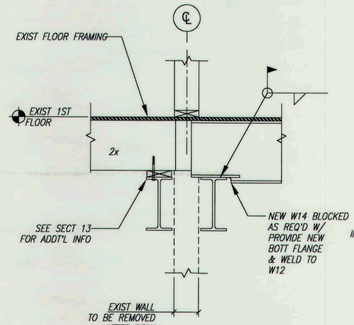
6/S7 TYPICAL SECTION
3/4" = 1'-0"



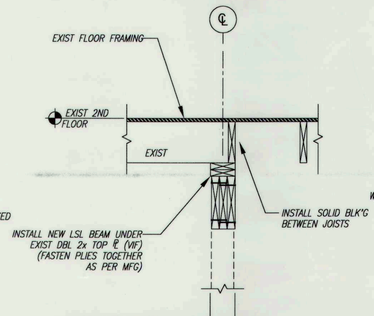
2/S7 TYPICAL SECTION
3/4" = 1'-0"



19/S7 TYPICAL SECTION
3/4" = 1'-0"



15/S7 TYPICAL SECTION
3/4" = 1'-0"



11/S7 TYPICAL SECTION
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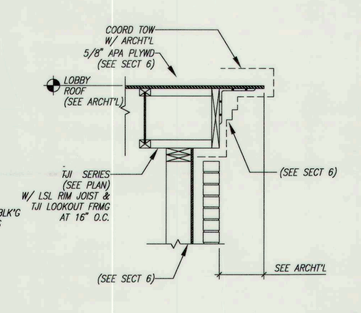
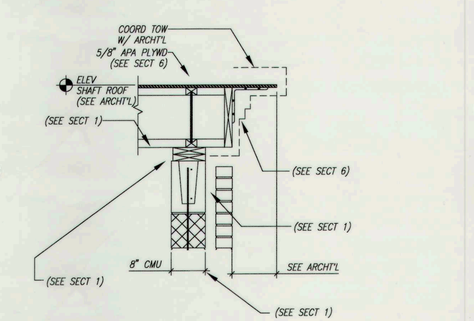
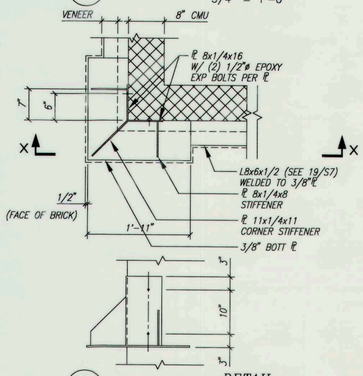


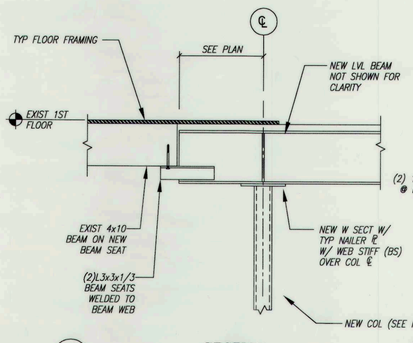
7/S7 TYPICAL SECTION
3/4" = 1'-0"



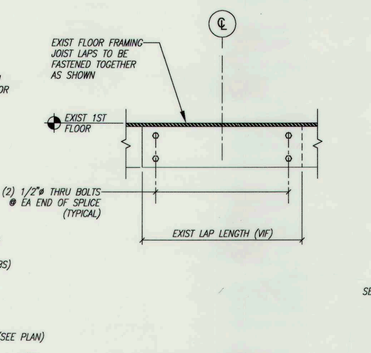
3/S7 SECTION
3/4" = 1'-0"



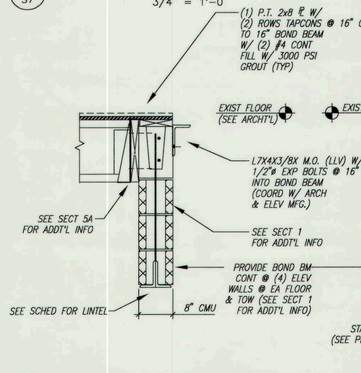
20/S7 DETAIL
3/4" = 1'-0"



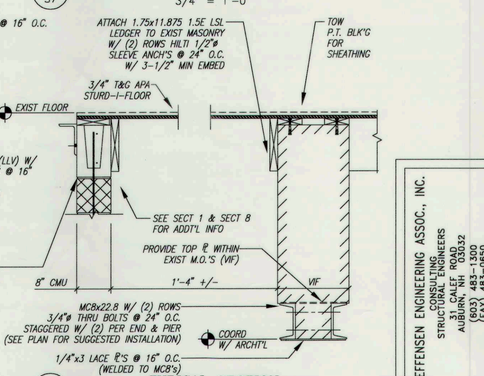
16/S7 SECTION
3/4" = 1'-0"



12/S7 TYPICAL DETAIL
3/4" = 1'-0"



8/S7 SECTION @ ELEVATOR
3/4" = 1'-0"



4/S7 TYPICAL SECTION
3/4" = 1'-0"

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REV. NO.	REVISIONS	DATE

PROJECT:
TOWN OF DERRY
ADAMS MEMORIAL BUILDING RENOVATIONS
DERRY NEW HAMPSHIRE

SUBJECT:
FRAMING SECTIONS & DETAILS

SCALE: AS NOTED

DATE: 06/09/00

UDELSMAN ASSOCIATES
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161 FEDERAL HILL ROAD
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PROJECT NO.:
99-064

SHEET NO.:
S7