GENERAL:

WOOD:

- Structural drawings shall be used in conjunction with the architectural, mechanical, electrical and shop drawings, and specifications.
- Unless otherwise noted, sections, details, notes, materials, and methods shown on any drawings are to be considered typical for all similar conditions.
- In the event of a conflict between plans, specifications, and details, the Structural Engineer shall be notified immediately for clarification.
- Due to minimal selective demolition, the existing framing conditions are not fully defined and will require field verification.
- All dimensions, elevations, and conditions must be verified in the field by the Contractor. Any discrepancies between these drawings and as-built conditions shall be brought to the attention of the Structural Engineer before proceeding with any work.
- The structure has been designed to be self-supporting and stable after the work shown on these drawings has been completed. The Contractor shall be responsible for the stability of the structure prior to the completion of work including but not limited to, jobsite safety, all shoring, bracing, erection methods, erection sequence, and forms required during construction. Temporary supports required for stability during all intermediate stages of construction shall be designed, furnished, and installed by the Contractor.
- The Contractor shall provide and maintain shoring and bracing supports as required to preserve stability and prevent movement, settlement, or collapse of adjacent construction to remain.
- All shoring and bracing shall be designed and certified by a professional engineer licensed in the jurisdiction of the project. Submittals to the Structural Engineer of all shoring and bracing for review and approval are required prior to the start of construction.
- Shop drawings shall be submitted to the Structural Engineer (see each section for specific items and requirements). Fabrication shall not proceed until a satisfactory review is received, the Contractor is proceeding at their own risk if failure to do so. Erection shall be executed from final reviewed shop drawings only.
- 10. Items noted on drawings as "by others" or "designated for design by others" indicates design and supply of structural items not by TFM. These items are a designated design item that shall be submitted for approval.
- Loads, openings, and structure relating to other non-structural disciplines are shown for bidding purposes only. Refer to architectural and mechanical drawings for the full scope of work.
- 12. These plans were prepared under the supervision of a licensed professional engineer. TFMoran Inc. assumes no liability as a result of any changes or non-conformance with these plans except upon the written approval of the Engineer of Record.
- 13. TFMoran Inc. assumes no liability for work performed without an acceptable program of testing and inspection as approved by the Engineer of Record.
- Reproduction of structural drawings for shop drawings is not permitted. Electronic drawing files will not be provided to the Contractor unless a transfer agreement has been completed between the Structural Engineer and the Contractor.
- 15. All work shall comply with the building codes referenced on these drawings.
- 16. Do not scale drawings. Contact the Architect or Structural Engineer for dimensions not specifically shown.

CODE:

2015 International Existing Building Code as amended, altered, or deleted by the provisions of the New Hampshire State Building Code.

DESIGN LOADS

MINIMUM UNIFORM LIVE LOADS AND MINIMUM CONCENTRATED LIVE LOADS: OCCUPANCY or USE UNIFORM CONCENTRATED Assembly Areas and Theaters Stage (reduced): 100 psf N/A CONCENTRATED FLOOR LOADS: If listed above, the concentrated load shall be used to determine the greatest load effect. Unless otherwise specified, the indicated concentration shall be assumed to be uniformly distributed over an area of 2.5 feet square and located to produce the max. load effects. ROOF SNOW LOAD: Risk Category: Ground Snow Load, Pg : 65 psf at 600 ft Allowed Reduction per ERDC/CRREL TR-02-6: 0.01*(600-300)*2.1=6.3 psf Ground Snow Load per ERDC/CRREL TR-02-6: 58.7 psf at 300 ft Snow Load Importance Factor, Is: 1.0 Snow Exposure Factor, Ce: 1.0 Thermal Factor, Ct: 1.0 Flat Roof Snow Load, Pf: 41.1 psf Drifting, sliding, and unbalanced snow loads: Per ASCE-7 Rain loads: Per ASCE-7 Roof live load: 20 psf MIN DEAD LOAD: Supported / Elevated Floor dead load: Design of stage framing members: 15 psf Design of ceiling framing members: 15 psf WIND DESIGN DATA: Wind loads have been determined using ASCE-7 Method 1 Simplified Procedure. Risk Category: Ultimate Wind Speed (3 second gust), Vult: 120 mph Wind Exposure Category: Internal Pressure Coefficient: 0.18 Components and Cladding Design Wind Pressure: MAX Negative (20 sf) Zone Per ASCE-7 MAX Positive (20 sf 16.0 psf 32.6 psf 16.0 psf 38.1 psf 16.0 psf 64.4 psf 25.9 psf 28.1 psf 33.8 psf 25.9 psf EARTHQUAKE DESIGN DATA: Earthquake Design for Existing Buildings: Not required since the proposed additions/alterations do not increase the force in any structural element by more than 5 percent nor do they decrease the strength of any structural element to less than required by the building code for new structures.

- Work shall be in accordance with the applicable American Wood Council, ANSI / AF&PA, "National Design Specification for Wood Construction (NDS)" including "Design Values for Wood Construction", National Forest Products Association.
- New wood for structural use shall have a moisture content as specified in the "National Design Specification for Wood Construction."
- Wood construction shall conform to applicable IBC, Chapter and Section for "Conventional Light-frame Construction."
- Sheathing panels shall be marked with the American Plywood Association (APA) trademark and shall meet the latest U.S. Product Standard PS 1 or APA PRP-108 Performance Standards.
- All wall sheathing panels shall be 1/2" thick Plywood or OSB, APA Rated Sheathing EXP 1 (C-D for Plywood) or (DOC PS1 or PS2 for OSB), with 32/16 span rating (unless otherwise noted). All sheathing panel edges shall be blocked, unless otherwise noted. Fasten with 8d common nails spaced at 6"o.c. at panel perimeter supported edges and 12"o.c. at interior intermediate supports (field) with 1 3/8" min. fastener penetration, unless otherwise noted. Lay wall sheathing with long dimension perpendicular to support members.
- All floor sheathing panels (sub-floor) shall be 3/4" thick Plywood or OSB, APA Rated Sturd-I-Floor EXP 1 (C-D for Plywood) or (DOC PS1 or PS2 for OSB), with 48/24 span rating (unless otherwise noted). Sheathing to be glued with adhesives meeting APA Spec. AFG-01 and fastened with 8d common nails spaced at 6"o.c. at panel perimeter supported edges and 12"o.c. at interior intermediate supports (field) with 1 3/8" min. fastener penetration, unless otherwise noted. Lay floor sheathing with long dimension perpendicular to support members and stagger sheathing panels in a row one half panel length with previous row. [Sheathing to be fastened to cold-formed metal framing with flathead teks 8-18 x 1-5/16" at 6" o.c. along panel edges and 12" o.c. at intermediate supports.
- Framing for walls, joists, rafters, beams and headers shall be Spruce-Pine-Fir (SPF) No. 1/ No. 2, unless noted. Dimensioned lumber represents nominal sizes. See minimum properties below:
- Wood exposed to the weather or in contact with concrete or masonry shall be pressure treated (P.T.) Southern Pine No. 1, unless noted. See minimum properties below:
- Laminated Veneer Lumber (LVL) members shall be 1.9E Trus Joist Microllam LVL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below:
- Parallel Strand Lumber (PSL) members shall be 2.0E (Beam) or 1.8E (Columns) Trus Joist 10. Parallam PSL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below.
- Laminated Strand Lumber (LSL) joists and stud members shall be 1.3E Trus Joist TimberStrand LSL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below.
- Laminated Strand Lumber (LSL) beam and header members shall be 1.55E Trus Joist 12. TimberStrand LSL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below.

3.	Wood framing shall have the minimum design values:			
		Min. Design Values		
	Species / Material	E (psi)	Fb (psi)	
	Spruce-Pine Fir (SPF) No. 1/ No. 2:	1.4e6	875	
	Southern Pine (SP) No. 1:	1.4e6	1,100	
	Laminated Veneer Lumber (LVL) 1.9E members:	1.9e6	2,600	
	Parallel Strand Lumber (PSL) 2.0E (Beams):	2.0e6	2,900	
	Parallel Strand Lumber (PSL) 1.8E (Columns):	1.8e6	2,400	
	Laminated Strand Lumber (LSL) 1.3E (Joists / Studs):	1.3e6	1,700	
	Laminated Strand Lumber (LSL) 1.55E (Beams/ Headers):	1.55e6	2,325	

14. Pressure treated (P.T.) wood shall meet the following standards for each condition of use: Min Pressure

Condition	Treatment	Retention	Category
Interior Construction:			
(Wood not exposed to weather,	CCA, ACQ-C,D	.25	UC2
in contact with concrete or masonry)	CA-B	.1	UC2
	MCA-C	.05	UC2
Above Ground, exterior construction:			
(Beams, joists and stringers	CCA, ACQ-C,D	.25	UC3B
not in contact with the ground)	CA-B	.1	UC3B
	MCA-C	.05	UC3B
Ground Contact, fresh water:			
(Posts and members exposed to weather	CCA, ACQ-C,D	.4	UC4A
and in ground contact)	CA-B	.21	UC4A
- /	MCA-C	.15	UC4A

Treated Sheathing

Chromated Copper Arsenate (CCA), Alkaline Copper Quaternary (ACQ), Copper Azole (CA) and Micronized Copper Azole (MCA)

- Field treat cut ends of P.T. wood with Copper Naphthenate preservative such as Copper-Greene.
- 15. Wood to steel and wood to wood bolted connectors shall be made with ASTM A307 bolts with flat washers. Bolt holes in wood shall be 1/32" larger than the bolt. Wood nailers shall be fastened with (2) rows of 1/2" diameter bolts staggered at 2'-0" o.c. unless otherwise noted.
- 16. Anchor bolts for wood sill plates to concrete shall be min. ASTM A307 headed or hooked bolts of the diameters and dimensions detailed or noted on the drawings.
- 17. Fastening Schedule: Plate to Stud, Direct 2-16d Stud to Plate, Toenail 4-8d
 - SEE APPLICABLE IBC TABLE "FASTENING SCHEDULE" FOR FASTENING/ NAILING REQUIREMENTS NOT SHOWN.
- 18. The lateral bracing system includes plywood wall and roof sheathing. Contractor shall provide temporary bracing as required to laterally support the structure during construction.
- 19. Provide lateral support at all bearing points and along compression edges at intervals of 24"o.c. or closer.
- 20. Minimum section width = $1 \frac{3}{4}$ ". The $3 \frac{1}{2}$ ", $5 \frac{1}{4}$ ", and 7" members may be combinations of 1 3/4" members. Follow manufacturers guidelines for Multiple Member Connections for side loaded beams.
- 21. Wood Construction Connectors shall be manufactured by Simpson Strong-Tie Co., MiTek Industries, Inc. or approved equal and installed in accordance with the manufacturers recommendations.
- 22. All wood fasteners and hangers in contact with pressure treated (P.T.) and or fire retardant treated (FRT) lumber are to be stainless steel or hot dipped galvanized (min 2.0 oz/ft^2). Hangers located in Ocean/Water Front environments shall be stainless steel.
- 23. All non-load bearing / non-structural walls shall be held down from joists / rafters / trusses above.
- 24. The Contractor shall retain the service of a certified lumber grader to identify the existing wood members that have rot, insect damage, severely cracked, checked, split or otherwise have deteriorated to the point that it needs to be either reinforced or replaced. Existing conditions to be identified before proceeding with any work.
- 25. Shop or Erection (placement) drawings shall be submitted to the Structural Engineer showing the ayout, sizes and anchorage details for all engineered lumber framing.
- 26. Submittals to the Structural Engineer shall include engineered lumber and hanger / connections product data, engineered lumber product data.

STRUCTURAL STEEL:

Fabricate and erect structural steel in accordance with the applicable "Specification for Structural 1. Steel for Buildings" and the "Code of Standard Practice" of AISC. Welding shall conform to the requirements of the "Structural Welding Code" of the American Welding Society. Structural steel wide flange shapes shall conform to ASTM A992 (Fy = 50 ksi). Hollow Structural Sections (HSS) shall conform to ASTM A500, Grade B (Rectangular Fy = 46 ksi, Round Fy = 42 ksi). Pipe shall be ASTM A53, Grade B (Fy = 35 ksi). Structural steel channels, misc. shapes, plates. and angles shall conform to ASTM A36 (Fy = 36 ksi), unless otherwise noted. Do not splice structural steel members without written approval of the Structural Engineer. Bolted connections shall be made with three-guarter inch diameter high strength. ASTM A325-N bolts, unless otherwise noted. Connections at moment frames, braced frames, column splices and hangers shall be made with three-quarters inch diameter A325-SC (Slip critical) bolts, unless otherwise noted. Shop connections, unless otherwise noted, shall be welded. Unless otherwise noted, beam connections shall provide shear capacity to support a reaction R equal to half the total uniform load capacity of the beam for given shape, span and steel specification (AISC) taking account for the effect of concentrated loads. Shop camber steel beams as shown on the drawings. Camber tolerance shall be -0" or +1/2". Camber shall be measured with beam web vertical (erected condition) under its own dead load. Anchor bolts shall be headed bolts of the diameters and dimensions detailed, unless otherwise noted on the drawings. Provide ASTM F1554 Gr. 36 for diameters 3/4" or less. Anchor bolts shall be set by template. Hooked ("J" type) anchor bolts are not permitted. Welding electrodes shall conform to AWS A5.1 E70XX series with proper rod to produce optimum weld (low hydrogen). Unless otherwise noted, bolted connections with slotted holes shall be field-welded with one-quarter inch fillet welds after final field adjustment. 10. Provide 3/8" minimum fitted stiffener plates each side at beams' web framing over columns and at beams supporting columns above.

11. Provide 1/4" thick steel leveling plate on 3/4" min. non-shrink grout under all column base plates unless otherwise noted. Leveling plates shall be set and grouted prior to erecting columns.

12. Provide all angles, plates, anchors, bolts, etc., shown on architectural drawings. 13. Lintels for exterior masonry and structural steel exposed to weather shall be hot-dip galvanized

according to ASTM A123.

14. Provide holes in beam top flanges for nailer attachment at steel supporting wood framing, see schedules, details and notes for size and spacing.

15. Steel Primer:

General Primer: Standard Alkyd Primer applied at 2.5 - 3.5 mil DFT shall be used as the standard of quality and performance. Color: grey

Touch up paint in the field by the Contractor, unless otherwise noted.

The Steel fabricator is responsible for the design and detailing of all connections including moment connections, braced frame connections and beam and/or column stiffeners and doublers if required. All connections must be designed by a registered professional engineer in the state in which the project is being constructed. Certification of this design shall be provided with the shop drawings submittal for review by the engineer of record.

17. Shop or Erection drawings shall be submitted to the Structural Engineer for fabrication and erection of structural steel, prior to fabrication.

18. Submittals to the Structural Engineer are required for mill tests and sealed connection calculations.

STRUCTURAL TESTS AND INSPECTIONS:

- Structural Tests, Inspections, and Reports for steel construction and other applicable construction shall be promptly submitted in writing to the Structural Engineer and Contractor.
- Tests and Inspections shall be completed in accordance with the applicable IBC Special Inspection chapter. Due to the minor nature of the work and low wind and seismic forces, special inspections are not required however, the following tests, inspections, and observations are required.
- Remove and replace work where test results indicate that it does not comply with specified requirements. Additional testing and inspecting, at Contractor's expense, will be performed to determine compliance of replaced or additional work with specified requirements.

Wood Construction Inspections: Inspection of the wood construction shall be performed according to the following requirements:

Wood Framing: Notify Engineer of Record to periodically observe wood structural panel sheathing size, configuration, blocking, and fastening (including spacing, diameter, and length of nails or staples) and verify compliance with the Contract Documents.

Structural Steel by independent testing agency and/ or structural engineer:

Inspect steel framing and connection installation.

Structural Observations:

EOS = EDGE OF SLAB

ERV = ENERGY RECOVERY UNIT

E. / EX. / EXIST. = EXISTING

EQ. = EQUAL

EW = EACH WAY

EXT. = EXTERIOR

EXP. = EXPANSION

Notify engineer of progress of construction for coordination of site observations as defined in Chapter 17 of the International Building Code (IBC). These observations are intended for review of general design intent and do not relieve the general contractor of their responsibility to perform quality control.

ABBREVIATIONS	<u>ABBREVI</u>
2x = 2" NOMINAL THICK LUMBER AB = ANCHOR BOLT AFF = ABOVE FINISH FLOOR ALT. = ALTERNATE ALUM. = ALUMINUM APPROX. = APPROXIMATE	FFE = FINISHE FIN. = FINISHEI FLR. = FLOOR FNDN. = FOUNI FT. = FEET FTG. = FOOTIN
ARCH. = ARCHITECTURAL B/, B/O = BOTTOM OF BCX = BOTTOM CHORD EXTENSION B.I = BAR JOIST	GA. = GAUGE GALV. = GALVA GC = GENERAL GEOTECH. = G
BLDG. = BUILDING BM. = BEAM BOT. = BOTTOM	HDG = HOT DIF HORIZ. = HORIZ HSS = HOLLOW
BP = BASE PLATE BRG. = BEARING BRP = BEARING PLATE BS = BRICK SHELF BTW = BETWEEN	IBC = INTERNA IF = INSIDE FAC IN. = INCH INT. = INTERIO
CANT. = CANTILEVER CFS = COLD FORMED STEEL CIP = CAST IN PLACE	JNT. = JOINT JP = JOIST BEA JST. = JOIST
CJ = CONTROL JOINT	K = KIP
CL. = CENTERLINE CLR. = CLEAR CMU = CONCRETE MASONRY UNIT COL. = COLUMN CONC. = CONCRETE CONST. = CONSTRUCTION CONT. = CONTINUOUS COORD. = COORDINATE CTR. = CENTER	LB = POUND LGM = LIGHT G LLH = LONG LE LLV = LONG LE LONG. = LONG L.P. = LOW POI LP = LEVELING LSL = LAMINAT LVL = LAMINAT
DBL = DOUBLE Ø / DIA. = DIAMETER DIM. = DIMENSION DIST. = DISTANCE DJ = DOUBLE JOIST DK = DECK DN. = DOWN DWGS. = DRAWINGS	MANUF. = MAN MAX. = MAXIMU MECH. = MECHAN MIN. = MINIMUN ML = MASONR MO = MASONR MPH = MILES P
EA. = EACH EF = EACH FACE	MAS. / MSNRY. MTL. = METAL
ELEC. = ELECTRICAL ELEC. = ELECTRICAL ELEC. = ELECTRICAL ELEV. = ELECTRICAL	NIC = NOT IN C # / No. = NUMB NTS = NOT TO
EMBED. = EMBEDMENT ENG = ENGINEER EOD = EDGE OF DECK EOR = ENGINEER OF RECORD	OC / o.c. = ON O OF = OUTSIDE OPNG. = OPEN OSB = ORIENT

Fv (psi)

135

175

285

190

400

310

AWPA

UC4A

290

IATIONS (cont'd)

ED FLOOR ELEVATION

NDATION

ANIZED _ CONTRACTOR GEOTECHNICAL

PPED GALVANIZED IZONTAL W STRUCTURAL SECTION ATIONAL BLDG. CODE

ARING PLATE

GAUGE METAL EG HORIZONTAL EG VERTICAL ITUDINAL DINT 9 PLATE TED STRAND LUMBER TED VENEER LUMBER NUFACTURER IUM IANICAL NICAL, ELECTRICAL, PLUMBING RY LINTEL RY OPENING PER HOUR '. = MASONRY CONTRACT/SCOPE O SCALE CENTER

E FACE NING OSB = ORIENTED STRAND BOARD

ABBREVIATIONS (cont'd

PAF = POWDER ACTUATED FASTENER PC = PRECAST PE = PROFESSIONAL ENGINEER PEMB = PRE-ENGINEERED METAL BLDG. PL. = PLATE PLF = POUNDS PER LINEAR FOOT PRE-ENG. = PRE-ENGINEERED PSF = POUNDS PER SQUARE FOOT PSI = POUNDS PER SQUARE INCH PSL = PARALLAM STRAND LUMBER PT = PRESSURE TREATED PWD. = PLYWOOD RAD. = RADIUS REC. = RECOMMENDATION REINF. = REINFORCING / REINFORCE(D) REQ'D. = REQUIRED REV. = REVISION RF = ROOF RO = ROUGH OPENING RTU = ROOF TOP UNIT SCHD. = SCHEDULE SE = STRUCTURAL ENGINEER SF = SQUARE FEET SIM. = SIMILAR SPEC. = SPECIFICATION STD. = STANDARD STIFF. = STIFFENER / STIFFEN(ED) STL. = STEEL STRUCT. = STRUCTURAL TBD = TO BE DETERMINED T/, T/O = TOP OF ... TCX = TOP CHORD EXTENSION THK. = THICK TJ = TIE JOIST T/O BS, TOBS = TOP OF BRICK SHELF T/O STL, TOS = TOP OF STEEL T/O WALL, TOW = TOP OF WALL TRANS. = TRANSVERSE TYP. = TYPICAL U/S = UNDERSIDE UNO = UNLESS NOTED OTHERWISE VB / VR = VAPOR BARRIER / RETARDER VERT. = VERTICAL VIF = VERIFY IN FIELD W/ = WITH W/O = WITHOUT WD. = WOOD WL. = WALL WK. PT. = WORK POINT WS. = WATERSTOP WWF / WWM = WELDED WIRE FABRIC / MESH







2 PARTIAL FIRST FLOOR FRAMING PLAN 1/4" = 1'-0"

FRAMING PLAN NOTES

- DO NOT SCALE THIS DRAWING.
 THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION.
- 3. TOP OF FIRST FLOOR SHEATHING ELEVATION = 9'-10" (V.I.F.).
- 4. ALL FRAMING SHOWN IS TO BE FLUSH FRAMED WITH PREFABRICATED LIGHT GAGE HANGERS UNLESS OTHERWISE NOTED AS DROPPED. 5. UNLESS FASTENED WITH HANGERS TO A FLUSH HEADER/BEAM, INSTALL SOLID 2x BLOCKING
- BETWEEN RAFTERS/ TRUSSES OVER BEARING WALLS OR DROPPED BEAMS. 6. COORDINATE SIZE AND LOCATION OF ALL FLOOR PENETRATIONS WITH ARCHITECTURAL AND
- MEP DRAWINGS, PROVIDE SUPPLEMENTAL FRAMING AROUND OPENINGS. 7. PROVIDE BUILT UP 2x HEADERS WITH A MINIMUM OF 1 JACK AND 1 KING STUD FOR ALL WALL
- OPENINGS GREATER THAN 14" WIDE. ALL BUILT UP HEADERS TO BE SHIMMED FULL LENGTH WITH 1/2" SHEATHING TO MATCH WALL STUD THICKNESS.
- 8. ALL POSTS TO BE CONTINUOUS TO FOUNDATIONS U.N.O. PROVIDE SOLID BLOCKING AND OR SQUASH BLOCKS AT RIM JOISTS AND INTERMEDIATE BEARING POINTS OVER DROPPED BEAMS. 9. PROVIDE JOIST/RAFTER BRIDGING AT 8'-0"O.C. MAX.
- 10. ALL JOISTS/RAFTERS TO ALIGN WITH INTERIOR AND EXTERIOR WALL STUDS.
- 11. "F-1" FLOOR CONSTRUCTION: 3/4" T&G ADVANTECH OSB WOOD SHEATHING + 1" MAPLE BOARD. 12. INSTALL WEB STIFFENERS/SQUASH BLOCKS IN I-JOISTS THAT SUPPORT BEARING WALLS ABOVE
- IN ACCORDANCE WITH I-JOIST MANUFACTURER'S SPECIFICATIONS
- 13. PROVIDE MINIMUM OF (3) 2x POSTS IN WALLS AT BEAM BEARING LOCATIONS.

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		48 Constitution Drive	Bedford, NH 03110	Phone #: (603) 472-4488	Fax #: (603) 472-9747	www.ttmoran.com	I FM Project #: 16/83.30		n ot be copied, duplicated, replicated or ut the prior written permission of TFM.	signed by a duly authorized officer of TFM.
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FINISHES AS REQUIERD TO PERFORM STRUCTURAL WORK. NEW/REPLACED FINISHES TO MATCH EXISITNG.



2 STAGE FRAMING PLAN

FRAMING PLAN NOTES

- 1. DO NOT SCALE THIS DRAWING.
- 2. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION.
- 3. TOP OF FIRST FLOOR SHEATHING ELEVATION = 24'-4" (V.I.F.).
- 4. ALL FRAMING SHOWN IS TO BE FLUSH FRAMED WITH PREFABRICATED LIGHT GAGE HANGERS UNLESS OTHERWISE NOTED AS DROPPED.
- 5. UNLESS FASTENED WITH HANGERS TO A FLUSH HEADER/BEAM, INSTALL SOLID 2x BLOCKING
- BETWEEN RAFTERS/ TRUSSES OVER BEARING WALLS OR DROPPED BEAMS. 6. COORDINATE SIZE AND LOCATION OF ALL FLOOR PENETRATIONS WITH ARCHITECTURAL AND
- MEP DRAWINGS, PROVIDE SUPPLEMENTAL FRAMING AROUND OPENINGS. PROVIDE BUILT UP 2x HEADERS WITH A MINIMUM OF 1 JACK AND 1 KING STUD FOR ALL WALL
- OPENINGS GREATER THAN 14" WIDE. ALL BUILT UP HEADERS TO BE SHIMMED FULL LENGTH WITH 1/2" SHEATHING TO MATCH WALL STUD THICKNESS.
- 8. ALL POSTS TO BE CONTINUOUS TO FOUNDATIONS U.N.O. PROVIDE SOLID BLOCKING AND OR SQUASH BLOCKS AT RIM JOISTS AND INTERMEDIATE BEARING POINTS OVER DROPPED BEAMS. 9. PROVIDE JOIST/RAFTER BRIDGING AT 8'-0"O.C. MAX.
- 10. ALL JOISTS/RAFTERS TO ALIGN WITH INTERIOR AND EXTERIOR WALL STUDS. "F-1" FLOOR CONSTRUCTION: 5/8" T&G ADVANTECH OSB WOOD SHEATHING +, 3/4" NEOPRENE
- BLOCKS+ 1/2" BIRCH FLOOR UNDERLAYMENT+ 3/4" T&G HARDWOOD. SEE DETAIL 3 THIS SHEET.
- INSTALL WEB STIFFENERS/SQUASH BLOCKS IN I-JOISTS THAT SUPPORT BEARING WALLS ABOVE
- IN ACCORDANCE WITH I-JOIST MANUFACTURER'S SPECIFICATIONS 3. PROVIDE MINIMUM OF (3) 2x POSTS IN WALLS AT BEAM BEARING LOCATIONS.



OWNER 1/2" BIRCH FLOOR AS APPROVED BY OWNER

HARDWOOD FLOORING AS APPROVED BY



43 03 44 09) (90 13) (90

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48 Cor Bedfor Phone Fax #: www.tf

ENGINEER

NEW

Maci

FURR OUR PARTITION WALL BELOW AND RETURN TO PERPENDICULAR WALL WITH GYPSUM WALL BOARD FINISH AND TRIM/MOLDING TO MATCH EXISTING AS REQUIRED, COORD WITH OWNER

STANDARD NAIL SIZE	DIAMETER	LENGTH	GUN NAIL SUBSTITUTES		
6d COMMON	0.113"	2"	-		
7d COMMON	0.113"	2 1/4"	-		
8d COMMON	0.131"	2 1/2"	(2) - 2 3/8" x 0.113" OR (2) - 2 1/4" x 0.099"		
	0.148"	3"	(2) - 2 1/2" x 0.113" TO (2) - 3 1/2" x 0.135"		
			(3) - 2 3/8" x 0.113" OR (3) - 2 1/4" x 0.099"		
	0.148"	3 1/4"	(2) - 2 3/8" x 0.113" OR (2) - 2 1/4" x 0.099"		
			(3) - 2 3/8" x 0.113" OR (3) - 2 1/4" x 0.099"		
	0.162"	3 1/2"	(2) - 2 1/2" x 0.131" TO (2) - 3 1/4" x 0.148"		
16d COMMON			(3) - 2 3/8" x 0.113" OR (3) - 2 1/2" x 0.113"		
			(4) - 2 1/4" x 0.099"		
NOTE: • FOR MORE INFORMATION REGARDING GUN NAIL SUBSTITUTES, SEE ESR-1539.					

5 STANDARD NAIL/GUN NAIL SIZES





WITH THE MOST CAPACITY.

NOTES:

4 TYPICAL BUILT-UP LVL BEAM CONNECTION SCHEDULE

-

-

22'-6"

-

-

-12'-4"

20'-0"

EWP-PLY

EWP-PLY

NOTE:

6 3/4" SIMPSON SDW (2) AT 24" o.c.

(2) AT 16" o.c.

(2) AT 16" o.c.

BE DOUBLED FOR 12" o.c. FASTENER SPACING.

1. SPAN VALUES FOR <u>NAILED PLYS</u> MAY BE DOUBLED FOR 6" o.c. SPACING OR TRIPLED FOR 4" o.c. SPACING.

SPAN VALUES FOR SCREWED PLYS FASTENED AT 24" o.c. MAY

IVIII			I
CONNECTION	MINIMUM FASTENING	MINIMUM FASTENING (GUN NAILS)	LOCATION
JOIST TO SILL OR GIRDER	(3)- 8d COMMONS	(3)- 3"x0.131"	TOENAIL
BRIDGING TO JOIST	(2)- 8d COMMONS	(2)- 3"x0.131"	TOENAIL EACH END
OLE PLATE TO JOIST OR BLOCKING	16d COMMONS AT 16"o.c.	3"x0.131" AT 12"o.c.	TYPICAL FACE NAIL
LE PLATE TO JOIST OR BLOCKING AT SHEAR WALL PANEL	(2)- 16d COMMONS AT 16"o.c.	(4)- 3"x0.131" AT 12"o.c.	SHEAR WALL PANELS
TOP PLATE TO STUD	(2)- 16d COMMONS	(3)- 3"x0.131"	END NAIL
STUD TO SOLE PLATE	(4)- 8d COMMONS	(4)- 3"x0.131"	TOENAIL
STUD TO SOLE PLATE	(2)- 16d COMMONS	(3)- 3"x0.131"	END NAIL
JBLE STUDS (NOT AT BRACED WALLS)	16d COMMONS AT 24"o.c.	3"x0.131" AT 16"o.c.	FACE NAIL
OUBLE STUDS (AT BRACED WALLS)	16d COMMONS AT 16"o.c.	3"x0.131" AT 12"o.c.	FACE NAIL
DOUBLE TOP PLATES	16d COMMONS AT 16"o.c.	3"x0.131" AT 12"o.c.	TYPICAL FACE NAIL
DOUBLE TOP PLATES	(8)- 16d COMMONS	(12)- 3"x0.131"	LAP SPLICE
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3)- 8d COMMONS	(3)- 3"x0.131"	TOENAIL
RIM JOIST TO TOP PLATE	8d COMMONS AT 6"o.c.	3"x0.131" AT 6"o.c.	TOENAIL
P PLATES, LAPS AND INTERSECTIONS	(2)- 16d COMMONS	(3)- 3"x0.131"	FACE NAIL
CONTINUOUS HEADER, TWO PIECES	16d COMMONS AT 16"o.c.	3"x0.131" AT 12"o.c.	FACE NAIL
CEILING JOIST TO PLATE	(3)- 8d COMMONS	(3)- 3"x0.131"	TOENAIL
CONTINUOUS HEADER TO STUD	(4)- 8d COMMONS	-	TOENAIL
LING JOISTS, LAPS OVER PARTITIONS	(3)- 16d COMMONS	(4)- 3"x0.131"	FACE NAIL
ILING JOISTS TO PARALLEL RAFTERS	(3)- 16d COMMONS	SEE IBC SECTION 2308.7.3.1	FACE NAIL
RAFTER TO PLATE	(3)- 16d COMMONS	(4)- 3"x0.131"	TOENAIL
DIAGONAL BRACE TO EACH STUD AND PLATE	(2)- 8d COMMONS	(2)- 3"x0.131"	FACE NAIL
BUILT-UP CORNER STUDS	16d COMMONS AT 24"o.c.	-	FACE NAIL
BUILT-UP GIRDER AND BEAMS	20d COMMONS AT 32"o.c.	3"x0.131" AT 24"o.c.	FACE NAIL AT TOP AND BOTTOM, STAGGERED ON OPPOSITE SIDES
BUILT-UP GIRDER AND BEAMS	(2)- 20d COMMONS	(3)- 3"x0.131"	FACE NAIL AT ENDS AND SPLICES
2" PLANKS	16d COMMONS	-	FACE NAIL EACH END
COLLAR TIE TO RAFTER	(3)- 10d COMMONS	(4)- 3"x0.131"	FACE NAIL
JACK RAFTER TO HIP	(3)- 10d COMMONS	(4)- 3"x0.131"	TOENAIL
JACK RAFTER TO HIP	(2)- 16d COMMONS	(3)- 3"x0.131"	FACE NAIL
ROOF RAFTER TO 2-BY RIDGE BEAM	(3)- 10d COMMONS	(4)- 3"x0.131"	TOENAIL
ROOF RAFTER TO 2-BY RIDGE BEAM	(2)- 16d COMMONS	(3)- 3"x0.131"	FACE NAIL
JOIST TO BAND JOIST	(3)- 16d COMMONS	(4)- 3"x0.131"	FACE NAIL
LEDGER STRIP	(3)- 16d COMMONS	(4)- 3"x0.131"	FACE NAIL AT EACH JOIST/STUD
WOOD STRUCTURAL PANELS TO FRAMING	8d COMMONS AT 6"o.c.	-	EDGE NAILING
WOOD STRUCTURAL PANELS TO FRAMING	8d COMMONS AT 12"o.c.	-	FIELD NAILING

 PROVIDE NAIL ABOVE UNO ON NOTES AND DETAILS, IF CONFLICT BETWEEN TABLE NOTES AND DETAILS EXISTING, PROVIDE CONNECTIONS SEE IBC TABLE 2304.10.1 FOR ADDITIONAL CONNECTIONS NOT LISTED.

1 MINIMUM WOOD FASTENING SCHEDULE



